



TECHNICAL MEMORANDUM

DATE: November 18, 2024
TO: Bally's Corporation
FROM: Peter Reinhofer, P.E., V3 Companies
RE: **Jefferson Street Traffic Volume Assessment**
Chicago, Illinois

V3 Companies has prepared this updated technical memorandum to present the conclusions of the peak hour analysis of Jefferson Street, a new north/south roadway that will generally provide one travel lane in each direction from Grand Avenue to Chicago Avenue. This new road will provide access to the proposed entertainment district access points and to the adjacent parcels for PD 1426 such as Huron Street, Erie Street, and Desplaines Street. The proposed development is located at the former Chicago Tribune printing plant property at 703 West Chicago Avenue.

The objective of this updated technical memorandum is to determine the capacity of Jefferson Street with the traffic generated by the proposed entertainment district and the full PD 1426 Build Out in 2032 during the weekday commuter and evening casino peak periods. The proposed entertainment district is anticipated to consist of the following:

- Approximately 184,000 square feet of casino area with a total of 4,000 gaming positions
- 500-room hotel
- Approximately 60,000 square feet of restaurant, retail, and bar space
- An entertainment venue with an approximate 3,000-seat theatre

In addition to the entertainment district, the remaining PD 1426 parcel is proposed to be developed with approximately 5,055 residential units with ancillary ground floor retail. The proposed development plan for the remaining parcels is based on the 2018 approved PD 1426 land use plan but may be modified in the future as development occurs.

This analysis of Jefferson Street will evaluate the potential impacts of the proposed entertainment district, which will open in 2026, and the full PD 1426 redevelopment which is anticipated to be built out by 2032.

Roadway System

Roadway Descriptions

Jefferson Street is a new north/south roadway that will generally provide one travel lane in each direction from Grand Avenue to the south to Chicago Avenue to the north. The southern intersection will be near the existing unsignalized intersection for the Tribune property and align with the public right of way on the south side of Grand Avenue. This intersection is proposed to be signalized as part of the entertainment district redevelopment project. The northern intersection will be located on Chicago



Avenue between Halsted Street and the bridge over the Chicago River. Figure 1 illustrates the proposed Jefferson Street geometrics.

Intersections

The intersection of *Jefferson Street and Parking Garage Driveway/Private Driveway* is planned to be a four-leg, signalized intersection. This intersection warrants a traffic signal based on the four-hour vehicular volume during a typical weekday. The ultimate configuration of the northbound approach consists of one left turn lane, one through lane, and one right turn lane while the southbound approach consists of one left turn lane and one shared through/right turn lane. The westbound approach of the parking garage driveway consists of one shared left turn/through lane and one right turn lane while the eastbound approach for the private driveway will consist of one shared left turn/through/right turn lane. Prior to the development of the west parcel, the northbound left turn lane will be striped out.

South of the parking garage signalized intersection is a right out only driveway from the casino drop off. It is anticipated that the casino drop off area will only be for valet drop off during peak times, therefore this right out only driveway will only be used by vehicles accidentally entering the valet area that do not wish to valet their vehicle.

The intersection of *Jefferson Street and Casino Lobby Driveway/Huron Street* is planned to be a four-leg, unsignalized intersection with eastbound and westbound stop-controlled approaches. The ultimate configuration of the southbound approach consists of one left turn lane and one shared through/right turn lane while the northbound approach consists of one left turn lane, one through lane, and one right turn lane. The eastbound approach consists of one shared left turn/through/right turn lane while the westbound approach consists of one departure lane that enters the casino valet drop off. Prior the development of the west parcels and the construction of Huron Street, the northbound left turn lane will be striped out.

The intersection of *Jefferson Street and Hotel Lobby Driveway/Erie Street* is planned to be a four-leg, unsignalized intersection with eastbound and westbound stop-controlled approaches. The ultimate configuration of the southbound and northbound approaches consists of one left turn lane and one shared through/right turn lane. The eastbound approach of Erie Street consists of one shared left turn/through/right turn lane while the westbound approach of the Hotel Lobby Driveway consists of one left turn lane and one shared through/right turn lane. Prior the development of the west parcels and the construction of Erie Street, the northbound left turn lane will be striped out.

The intersection of *Jefferson Street and South Parking Lot Driveway/Desplaines Street* is planned to be a four-leg, unsignalized intersection with eastbound and westbound stop-controlled approaches. The northbound approach consists of one shared left turn/through lane and one right turn lane while the southbound approach consists of one shared left turn/through lane and one shared through/right turn lane. The eastbound and westbound approaches consist of one shared left turn/through/right turn lane. It should be noted that there will likely be limited traffic to and from the existing parking lot on the east side of Jefferson Street during the Entertainment District only scenario. This driveway will also provide access to CDOT and IDOT to the Ohio Street Bridge House.



It should be noted that a set of ramps are also proposed along Jefferson Street north of the Desplaines Street/South Parking Lot Driveway that will provide access to the lower level of the Casino. This lower level will include the employee parking lot, truck docks, and a charter/casino bus pick up and drop off area. The lower level will be accessed with an inbound ramp from northbound Jefferson Street and with an outbound ramp to southbound Jefferson Street.

2026 Entertainment District Scenario

Trip Generation – 2026 Entertainment District

The proposed entertainment district redevelopment consists of a casino with hotel, restaurants, museum, and theatre. Project traffic is estimated using the Institute of Transportation Engineers (ITE) *Trip Generation Manual, 11th Edition*. The *Trip Generation Manual* assigns trip generation estimates based on either an average rate or a fitted curve equation for each peak period and an independent variable. In this case, the number of gaming positions is the applicable variable for a casino within an entertainment district. The peak hour trip generation equations are selected for the following time periods:

- weekday, am peak hour of generator for the weekday commuter am peak hour (7 – 9 am)
- weekday, peak hour of adjacent street traffic for one hour between 4 pm and 6 pm for the weekday commuter pm peak hour (4 – 6 pm)
- weekday, pm peak hour of generator for the Friday casino peak hour (8 – 11 pm)
- Saturday, peak hour of generator for the Saturday casino peak hour (8 – 11 pm)

The number of trips generated for the entertainment district was estimated based on the ITE data and reviewed for potential reductions for non-vehicular travel such as transit, walking, and capture between the casino and nearby existing and proposed residential, office, retail, and restaurants. Additionally, the number of taxi and rideshare trips was separately estimated as these trips generate entering and exiting trips for each pick up or drop off movement.

A conservative ten percent reduction was applied for non-vehicular traffic, including transit, walking, and biking for casino patrons and employees.

It is assumed that 55 percent of the trips will be personal vehicles that will drive and park in the casino parking lots. Of this traffic, five percent is assumed to be employee traffic that will park in the lower level and the remaining 50 percent will park in the parking garage. An additional 15 percent of trips will drive and utilize the valet parking available at both the Casino Lobby and Hotel Lobby driveways. Finally, it is assumed that 15 percent of trips will use rideshare and five percent will use taxis. A taxi and rideshare trip actually generates two trips, one entering the site to drop off or pick up and one exiting the site; therefore, a redundancy rate was also added to account for the exiting trip. Typically, a redundancy reduction rate is applied to these trips assuming that the taxi or rideshare driver will also wait for or pick up another passenger leaving the site. A redundancy reduction rate of 25 percent of the taxi and rideshare trips was assumed, meaning that one in four taxi and rideshare trips will drop off a patron and wait for and pick up another passenger without leaving the entertainment district area on Chicago Avenue or Grand Avenue.



Table 1 provides a summary of the trip generation for the entertainment district.

Table 1 – Entertainment District Trip Generation

LAND USE	SIZE		Weekday Commuter AM Peak			Weekday Commuter PM Peak			Friday Casino Peak			Saturday Casino Peak		
			In	Out	Total	In	Out	Total	In	Out	Total	In	Out	Total
ITE Land Use Code 473: Casino	4,000	Gaming Positions	676	511	1,187	867	834	1,701	1,149	1,061	2,210	1,474	1,256	2,730
<i>Local Area Capture Reduction (0%)</i>			0	0	0	0	0	0	0	0	0	0	0	0
<i>Non-Auto Trip Reduction (10%)</i>			-68	-51	-119	-87	-83	-170	-115	-106	-221	-147	-126	-273
<i>Subtotal Trips Generated</i>			608	460	1068	780	751	1531	1034	955	1989	1327	1130	2457
<i>Self-Drive and Park (55%)</i>			372	281	653	477	459	936	632	584	1216	811	691	1502
<i>Self-Drive and Valet (15%)</i>			101	77	178	130	125	255	172	159	332	221	188	410
<i>Taxi (5%)</i>			34	26	59	43	42	85	57	53	111	74	63	137
<i>Rideshare (15%)</i>			101	77	178	130	125	255	172	159	332	221	188	410
<i>Taxi / Rideshare Redundancy</i>			77	101	178	125	130	255	159	172	331	188	221	409
Total New Vehicle Trips			685	562	1,246	905	881	1,786	1,192	1,127	2,322	1,515	1,351	2,868

Trip Distribution and Assignment – 2026 Entertainment District

The directional distribution of new traffic volumes generated by the entertainment district is split between employee trips, valet trips, taxi/rideshare trips, and self-park trips, as these categories are expected to exhibit different distributions based on the proposed access points along Jefferson Street.

Employee trips are assumed to access the site to and from Grand Avenue via the ramping system within Jefferson Street. Additionally, any truck traffic and charter/casino buses along Jefferson Street will also utilize this ramping system.

Valet trips are assumed to have a 75 percent and 25 percent split between the north Casino Lobby Driveway and the south Hotel Lobby Driveway, respectively. Currently, the plan calls for the Hotel Lobby valet area to provide both drop off and pick up at the same location, so all inbound and outbound trips will travel through this driveway. For the Casino Lobby valet area, the drop off will occur at the casino valet area but the pick-up will occur within the parking garage, so exiting vehicles will travel through the parking garage and exit through the signalized parking garage intersection. Therefore, 75 percent of valet trips will enter through the Casino Lobby Driveway and exit through the Parking Garage Driveway to the north, and 25 percent of valet trips will enter and exit through the Hotel Lobby Driveway.

Taxi and rideshare trips are expected to use the Hotel Lobby Drop-off/Pick-Up area as well as the lay-by lanes along Jefferson Street. It is assumed that ten percent of the rideshare trips will utilize the Hotel Lobby Driveway, and the remaining 90 percent will utilize the passenger loading and unloading areas along the east and west side of Jefferson Street. It is anticipated that the lay-by lanes will be signed as passenger loading/unloading only with no parking and no waiting. The curb use will be enforced by traffic control aides during the peak periods of the casino.

Self-park vehicular trips are assumed to have a 90 percent and 10 percent split between the north Casino Lobby Driveway and the south Hotel Lobby Driveway, respectively. Currently, 90 percent of the inbound



and outbound self-park vehicular traffic is expected to utilize the parking garage access driveway approximately 360-feet south of the Jefferson Street and Chicago Avenue intersection and the remaining ten percent will use the Hotel Lobby Driveway.

The various entertainment district project trips are totaled to obtain the Entertainment District Site Traffic Volumes and illustrated in Figure 2. Figure 3 provides the Future with Entertainment District Traffic Volumes for the proposed opening of the casino for the four peak hours. It should be noted that the development on the north side of Chicago Avenue is not assumed to be constructed on the opening day of the entertainment district.

Traffic Signal Warrant Analysis – Entertainment District

Based on the projected traffic volumes at the intersection of Jefferson Street and Parking Garage Driveway, a traffic signal warrant analysis has been conducted. The investigation for the need for a traffic control signal is based on the methodology established in the Manual on Uniform Traffic Control Devices (MUTCD). The MUTCD establishes nine individual warrants. Installation of a traffic signal should be further investigated at locations that meet one or more warrants. However, warrant 1, the eight-hour vehicular volume warrant, and warrant 2, the four-hour vehicular warrant, are typically the primary vehicular volume/delay warrants that are considered for intersections.

Warrant 1 is met if a total of eight hours in the day exceed the thresholds established in the MUTCD. Traditionally, this warrant requires more than eight hours of data collection and substantial projections of future trips. Warrant 2 is met if a total of four hours in the day exceed the MUTCD thresholds. In order to estimate the four highest hours during a typical weekday, the weekday pm commuter peak hour and the Friday night casino peak hour were selected as two of the hours. Taking those two peak hours plus 95 percent of each hour to estimate one hour adjacent those peak hours were utilized in the warrant analysis. The IDOT methodology also requires a reduction of the minor approach right turn volume based on factors such as lane configuration and conflicting volumes.

Using the IDOT methodology to estimate the eighth-highest hour of traffic, the intersections of Jefferson Street and Chicago Avenue and Jefferson Street and Grand Avenue meet Warrant 1, Eight-Hour Vehicular Volume. Based on the projected four highest hours using the weekday pm peak hour and the Friday night casino peak hour traffic volumes, the intersection of Jefferson Street and Parking Garage Driveway meets Warrant 2, Four-Hour Vehicular Volume. The supporting Signal Warrant Review Sheet and Right Turn Factor Sheet is attached.

Capacity Analysis – Entertainment District

The operation of a facility is evaluated based on level of service (LOS) calculations obtained by analytical methods defined in the Transportation Research Board's *Highway Capacity Manual (HCM)*, 7th Edition. The concept of LOS is defined as a quality measure describing operational conditions within a traffic stream, generally in terms of such service measures as speed and travel time, freedom to maneuver, traffic interruptions, and comfort and convenience.



There are six LOS letter designations, from A to F, with LOS A representing the best operating conditions and LOS F the worst.

The LOS of an intersection is based on the average control delay per vehicle. For a signalized intersection, the delay is calculated for each lane group and then aggregated for each approach and for the intersection as a whole. Generally, the LOS is reported for the intersection as a whole. For an unsignalized intersection, the delay is only calculated and reported for each minor movement. An overall intersection LOS is not calculated.

There are different LOS criteria for signalized and unsignalized intersections primarily due to driver perceptions of transportation facilities. The perception is that a signalized intersection is expected to carry higher traffic volumes and experience a greater average delay than an unsignalized intersection. The LOS criteria for signalized and unsignalized intersections are provided in Table 2.

Typically, various state and local governments adopt standards varying between LOS C and LOS E, depending on the area’s size and roadway characteristics. Capacity analysis was performed with Synchro 11.1. Models were created for the weekday commuter am, weekday commuter pm, Friday evening casino, and Saturday evening casino peak hours for the future with Entertainment District scenario.

Table 2 – Level of Service Definitions for Signalized and Unsignalized Intersections

Level of Service	Signalized Intersection Control Delay (seconds/vehicle)	Unsignalized Intersection Control Delay (seconds/vehicle)
A	≤ 10	≤ 10.0
B	> 10.0 and ≤ 20.0	> 10.0 and ≤ 15.0
C	> 20.0 and ≤ 35.0	> 15.0 and ≤ 25.0
D	> 35.0 and ≤ 55.0	> 25.0 and ≤ 35.0
E	> 55.0 and ≤ 80.0	> 35.0 and ≤ 50.0
F	> 80.0	> 50.0

Source: Transportation Research Board, *Highway Capacity Manual 6th Edition*, National Research Council, 2016.

The study area includes the proposed signalized and unsignalized intersections along Jefferson Street and include the following:

- Jefferson Street & Parking Garage Driveway (signalized)
- Jefferson Street & Casino Lobby Driveway (unsignalized)
- Jefferson Street & Hotel Lobby Driveway (unsignalized)
- Jefferson Street & South Parking Lot Driveway/Desplaines Street (unsignalized)

The capacity analysis results at the study area intersections are summarized in Table 3.



Table 3 – Entertainment District: Capacity Analysis Results

Intersection / Approach	Weekday AM		Weekday PM		Friday Casino		Saturday Casino	
	Future w/ Project		Future w/ Project		Future w/ Project		Future w/ Project	
	Delay (s/veh)	LOS	Delay (s/veh)	LOS	Delay (s/veh)	LOS	Delay (s/veh)	LOS
Jefferson Street & Parking Garage Driveway (Signalized)								
EB Approach	-	-	-	-	-	-	-	-
WB Approach	17.9	B	20.9	C	19.1	B	23.3	C
NB Approach	5.5	A	8.1	A	16.4	B	31.3	C
SB Approach	5.5	A	2.5	A	17.0	B	6.5	A
Intersection	9.5	A	11.6	B	17.5	B	22.3	C
Jefferson Street & Casino Lobby Driveway (Unsignalized)								
SB Left	8.5	A	9.2	A	10.3	B	11.8	B
Jefferson Street & Hotel Lobby Driveway (Unsignalized)								
WB Right	11.8	B	14.6	B	20.7	C	37.2	E
SB Left	8.5	A	9.0	A	10.0	B	11.5	B
Jefferson Street & Desplaines Street/South Parking Lot Driveway (Unsignalized)								
NB Left/Thru	0.0	A	0.0	A	0.0	A	0.0	A
NB Thru/Right	0.0	A	0.0	A	0.0	A	0.0	A
EB Approach	0.0	A	0.0	A	0.0	A	0.0	A
WB Approach	0.0	A	0.0	A	0.0	A	0.0	A
SB Left/Thru	0.0	A	0.0	A	0.0	A	0.0	A
SB Thru/Right	0.0	A	0.0	A	0.0	A	0.0	A

Based on the capacity analysis results, all movements operate at LOS D or better during all four time periods with the exception of the westbound right turn movement at the Jefferson Street and Hotel Lobby Driveway which is projected to operate at LOS E during the Saturday casino peak hour.

In the event that any intersections or movements operate with high delays, it is recommended that traffic control aides may be needed to improve traffic flow during those times. Additional details regarding traffic control aides is provided in a later section of this Technical Memorandum.

Queue Analysis – Entertainment District

A queue length analysis was conducted at each of the study area intersections. A summary of this analysis is provided in Table 4. This data is comprised of the 95th percentile queue output from the Synchro analysis.



Table 4 – Entertainment District: 95th Percentile Queue Lengths

Intersection	Peak Period	95th Percentile Queue (feet)							
		Eastbound		Westbound		Northbound		Southbound	
		Left	Right	Left	Right	Left	Right	Left	Right
Jefferson Street and Chicago Avenue	Weekday Commuter AM Peak	-	11	92	-	144	243	-	-
	Weekday Commuter PM Peak	-	77	40	-	198	454	-	-
	Friday Evening Casino Peak	-	120	148	-	200	338	-	-
	Saturday Evening Casino Peak	-	94	222	-	258	639	-	-
Storage Length (ft)		-	55	95	-	190	190	-	-
Taper Length (ft)		-	55	90	-	75	75	-	-
Jefferson Street & Parking Garage Driveway (Signalized)	Weekday Commuter AM Peak	-	-	125	52	-	10	45	-
	Weekday Commuter PM Peak	-	-	198	59	-	12	17	-
	Friday Evening Casino Peak	-	-	205	132	-	45	142	-
	Saturday Evening Casino Peak	-	-	223	257	-	143	25	-
Storage Length (ft)		-	-	-	-	-	100	200	-
Taper Length (ft)		-	-	-	-	-	50	75	-
Jefferson Street & Casino Lobby Driveway (Unsignalized)	Weekday Commuter AM Peak	-	-	-	-	-	0	3	-
	Weekday Commuter PM Peak	-	-	-	-	-	0	3	-
	Friday Evening Casino Peak	-	-	-	-	-	0	5	-
	Saturday Evening Casino Peak	-	-	-	-	-	0	10	-
Storage Length (ft)		-	-	-	-	-	50	100	-
Taper Length (ft)		-	-	-	-	-	50	75	-
Jefferson Street & Hotel Lobby Driveway (Unsignalized)	Weekday Commuter AM Peak	-	-	-	10	-	-	3	-
	Weekday Commuter PM Peak	-	-	-	20	-	-	3	-
	Friday Evening Casino Peak	-	-	-	40	-	-	5	-
	Saturday Evening Casino Peak	-	-	-	85	-	-	10	-
Storage Length (ft)		-	-	-	-	-	-	100	-
Taper Length (ft)		-	-	-	-	-	-	50	-
Jefferson Street & Desplaines Street/South Parking Lot Driveway	Weekday Commuter AM Peak	0	-	0	-	-	0	-	-
	Weekday Commuter PM Peak	0	-	0	-	-	0	-	-
	Friday Evening Casino Peak	0	-	0	-	-	0	-	-
	Saturday Evening Casino Peak	0	-	0	-	-	0	-	-
Storage Length (ft)		-	-	-	-	-	85	-	-
Taper Length (ft)		-	-	-	-	-	100	-	-
Jefferson Street and Grand Avenue	Weekday Commuter AM Peak	37	-	-	-	-	-	96	32
	Weekday Commuter PM Peak	142	-	-	-	-	-	135	33
	Friday Evening Casino Peak	96	-	-	-	-	-	163	30
	Saturday Evening Casino Peak	155	-	-	-	-	-	187	27
Storage Length (ft)		100	-	-	-	-	-	100	100
Taper Length (ft)		100	-	-	-	-	-	180	180

The longest queues occur in the northbound direction at the Chicago Avenue and Jefferson Street intersection. It is anticipated that the queue lengths could back up into the parking garage intersection, blocking traffic from entering and exiting the driveway. It is recommended that traffic control aides be present to prevent northbound vehicles from blocking the driveway to maximize traffic flow through this



area. Additionally, this intersection should be synchronized with the Parking Garage Driveway to maximize efficiency between intersections. Shifting additional green time from Chicago Avenue to Jefferson Street would likely decrease the 95th percentile queue lengths along Jefferson Street. It is recommended that this area be monitored in the future to maximize the efficiency of Chicago Avenue and Jefferson Street.

The westbound right turn queue at the Jefferson Street and Garage Driveway intersection during the Saturday evening casino peak hour is 257 feet, or approximately ten vehicles. While the internal design and operations of the parking garage are still being finalized, particularly the locations of the gates for exiting vehicles, it is recommended that the exit gates be located more than 300 feet from this intersection to maximize the queue length such that queues from this intersection do not back up through the gates. If this does occur, it is recommended that traffic control aides provided by Bally's may be needed to improve traffic flow.

The analysis results indicate that there are no other significant queueing issues at the other study area intersections.

Figure 4 illustrates the proposed lane configuration at the study area intersections.

2032 PD 1426 Full Build Out

Trip Generation – 2032 PD 1426 Full Build Out

In addition to the entertainment district which is proposed to be constructed and operating by 2026, there are additional parcels within the study area that are proposed to be redeveloped. The proposed site plan for the remaining parcels of PD 1426 includes 13 new buildings which will primarily include residential dwelling units with the potential for ancillary ground floor retail and one hotel. Project traffic is estimated using the Institute of Transportation Engineers (ITE) *Trip Generation Manual, 11th Edition*.

The multifamily housing, high rise provides several subcategories and settings. While this site is within walking distance to the CTA Blue Line station and numerous CTA bus routes, the general urban/suburban setting and not close to rail transit subcategory were utilized. The U.S. Census Bureau publishes data that establishes mode splits for the area around the proposed redevelopment area using data from the 2020 American Community Survey. The data illustrates that the average drive alone split is 25.4 percent, carpooling is 0.9 percent, and taxi cab trips are 3.8 percent. The redundancy for the taxi trips was also assumed. The remaining commute trips included public transportation at 16.8 percent, biking and walking at 37.5 percent and 14.7 percent, respectively.

The peak hour trip generation equations are selected for the following time periods:

- weekday, am peak hour of generator for the weekday commuter am peak hour (7 – 9 am)
- weekday, peak hour of adjacent street traffic for one hour between 4 pm and 6 pm for the weekday commuter pm peak hour (4 – 6 pm)



Trip rates are not available for the Friday or Saturday casino peak hours so the vehicle time of day distribution from the *Trip Generation Manual* was used to estimate the trip generation for those time periods. For the Friday casino peak hour, the distribution percentage for the 8 pm to 9 pm hour, which is the casino peak hour, was compared to the 5 pm to 6 pm hour. The residential pm peak hourly distribution at 5 pm is 8.5 percent and at 8 pm is 5.2 percent. Similarly, the hourly distribution for the hotel at 5 pm is 7.7 percent and at 8 pm is 3.2 percent. It is assumed that the trip generation for the Friday casino peak hour is 61.2 percent of the weekday pm peak hour ($5.2 / 8.5$) for the multifamily housing and 41.6 percent ($3.2 / 7.7$) for the hotel.

A similar process was conducted to estimate the Saturday casino peak hour trips for the hotel. However, there is limited Saturday time of day distribution data for most residential housing land uses. The only land use with Saturday data was land use code 221, multi-family housing (mid-rise). It is assumed that hourly trip data for this land use will be similar to land use code 222. Saturday casino peak hour trips were calculated by taking the Saturday 8 pm hour and comparing to the Saturday peak hour of generator hourly distribution and applying that percentage to the Saturday peak hour trip generation.

It should be noted that there is potential for ground floor retail at the residential buildings but will likely be ancillary to the dwelling units and not major trip generating uses. ITE Land Use Code: 232 - High-Rise Residential with 1st-Floor Commercial was initially considered but after reviewing thoroughly, it was decided not to be used due to limited data points to estimate the trip rates, no directional distribution data, and no Saturday peak hour data.

Table 5 provides a summary of the trip generation for the PD 1426 parcels.



Table 5 – PD 1426 Trip Generation (Remaining Parcels)

PARCEL / Building Size	LAND USE	SIZE		Weekday Commuter AM Peak			Weekday Commuter PM Peak			Friday Casino Peak			Saturday Casino Peak		
				In	Out	Total	In	Out	Total	In	Out	Total	In	Out	Total
D-2.1/D-4.1/D-4.2 1.52 Mil Sq Ft	ITE Land Use Code: 222 Multifamily Housing (High-Rise)	1,500	Dwelling Unit	119	230	349	231	182	413	141	111	253	152	116	268
	Self-Drive Reduction (26.3%)			31	60	91	61	48	109	37	29	66	40	30	70
	Taxi (3.8%)			5	9	14	9	7	16	5	4	10	6	4	10
	Taxi Redundancy (3.8%)			9	5	14	7	9	16	4	5	9	4	6	10
	Ground Floor Retail*		30,000	SF	-	-	-	-	-	-	-	-	-	-	-
Parcel D-2.1/D-4.1/D-4.2 Total New Vehicle Trips				45	74	119	77	64	141	46	38	85	50	40	90
D-5.1 510,000 Sq Ft	ITE Land Use Code: 222 Multifamily Housing (High-Rise)	500	Dwelling Unit	44	85	129	86	67	153	53	41	94	57	44	101
	Self-Drive Reduction (26.3%)			12	22	34	23	18	40	14	11	25	15	11	26
	Taxi (3.8%)			2	3	5	3	3	6	2	2	4	2	2	4
	Taxi Redundancy (3.8%)			3	2	5	3	3	6	2	2	4	2	2	4
	Ground Floor Retail*		30,000	SF	-	-	-	-	-	-	-	-	-	-	-
Parcel D-5.1 Total New Vehicle Trips				17	27	44	29	24	52	18	15	33	19	15	34
C-1.1 500,000 Sq Ft	ITE Land Use Code: 222 Multifamily Housing (High-Rise)	495	Dwelling Unit	43	85	128	85	67	152	52	41	93	57	43	100
	Self-Drive Reduction (26.3%)			11	22	33	22	18	40	14	11	24	15	11	26
	Taxi (3.8%)			2	3	5	3	3	6	2	2	4	2	2	4
	Taxi Redundancy (3.8%)			3	2	5	3	3	6	2	2	4	2	2	4
	Ground Floor Retail*		5,000	SF	-	-	-	-	-	-	-	-	-	-	-
Parcel C-1.1 Total New Vehicle Trips				16	27	43	28	24	52	18	15	32	19	15	34
B-7.1/B-7.2 900,000 Sq Ft	ITE Land Use Code: 222 Multifamily Housing (High-Rise)	890	Dwelling Unit	73	142	215	143	112	255	87	69	156	94	71	166
	Self-Drive Reduction (26.3%)			19	37	56	38	29	67	23	18	41	25	19	44
	Taxi (3.8%)			3	5	8	5	4	10	3	3	6	4	3	7
	Taxi Redundancy (3.8%)			5	3	8	4	5	9	3	3	6	3	4	7
	Ground Floor Retail*		10,000	SF	-	-	-	-	-	-	-	-	-	-	-
Parcel B-7.1/B-7.2 Total New Vehicle Trips				27	45	72	47	38	86	29	24	53	32	26	58
B-4.1/B-5.1/B-5.2 1.075 Mil Sq Ft	ITE Land Use Code: 222 Multifamily Housing (High-Rise)	1,055	Dwelling Unit	85	166	251	166	131	297	102	80	182	111	83	194
	Self-Drive Reduction (26.3%)			22	44	66	44	34	78	27	21	48	29	22	51
	Taxi (3.8%)			3	6	9	6	5	11	4	3	7	4	3	7
	Taxi Redundancy (3.8%)			6	3	9	5	6	11	3	4	7	3	4	7
	Ground Floor Retail*		20,000	SF	-	-	-	-	-	-	-	-	-	-	-
Parcel B-4.1/B-5.1/B-5.2 Total New Vehicle Trips				31	53	84	55	45	100	34	28	62	36	29	65
B-1.1/B-1.2/B-2.1 1.095 Mil Sq Ft	ITE Land Use Code: 222 Multifamily Housing (High-Rise)	615	Dwelling Unit	52	102	154	102	81	183	62	50	112	68	52	120
	Self-Drive Reduction (26.3%)			14	27	41	27	21	48	16	13	29	18	14	32
	Taxi (3.8%)			2	4	6	4	3	7	2	2	4	3	2	5
	Taxi Redundancy (3.8%)			4	2	6	3	4	7	2	2	4	2	3	5
	Ground Floor Retail*		30,000	SF	-	-	-	-	-	-	-	-	-	-	-
Parcel B-1.1/B-1.2/B-2.1 Total New Vehicle Trips				50	81	131	57	58	115	30	29	59	33	24	57
Total New Vehicle Trips				186	307	493	293	253	546	175	149	324	189	149	338

* It is anticipated that the Ground Floor Retail will be ancillary to the proposed development and adjacent developments and will generate minimal primary vehicle trips

Trips Distribution and Assignment – 2032 PD 1426 Full Build Out

The remaining PD 1426 primarily consists of residential buildings with ground floor retail. Following the same methodology from the approved September 2018 *River District Traffic Impact Study* by Sam Schwartz, the direction distribution from which traffic approaches and departs a site is a function of numerous variables, including location of residences, location of employment centers, location of commercial/retail centers, available roadway systems, location and number of access points, and level of congestion on adjacent road systems. The approved 2018 study had different inbound and outbound percentages based on the roadway network. The overall trip distribution, including the street network distribution and the regional distribution is documented in the December 7, 2022 Traffic Impact Study



The proposed roadway network for the full PD 1426 redevelopment includes the construction of Huron Street from Halsted Street to Jefferson Street (south of the existing Huron Street alignment), Erie Street from Union Street to Jefferson Street, and Desplaines Street from Ohio Street to Jefferson Street. These roadway extensions are limited to being constructed after the abandonment of the Union Pacific Rail Spur since the UP requires a minimum height distance over the railroad tracks and discourages new at-grade rail crossings.

The trip assignment is based on the proposed roadway network for the full PD 1426 redevelopment, which includes the construction of the new east/west roadways. Figure 6 illustrates the site traffic for the remaining PD 1426 parcels. This includes vehicle trips on the new roadways for several of the PD 1426 parcels that will not travel on Jefferson Street. Figure 7 provides the overall PD 1426 site traffic with the entertainment district and remaining 13 buildings. Figure 8 provides the Future with Full PD 1426 Development Traffic Volumes for the full build out of the site. This includes traffic generated from the proposed development on the north side of Chicago Avenue.

Capacity Analysis – 2032 PD 1426 Full Build Out

Similar to the Entertainment District capacity analysis, Synchro was used to evaluate the study area intersections for the Full PD 1426 scenario. The capacity analysis results at the study area intersections are summarized in Table 6.



Table 6 – Full PD 1426 Build Out: Capacity Analysis Results

Intersection / Approach	Weekday AM		Weekday PM		Friday Casino		Saturday Casino	
	Future w/ Project		Future w/ Project		Future w/ Project		Future w/ Project	
	Delay (s/veh)	LOS	Delay (s/veh)	LOS	Delay (s/veh)	LOS	Delay (s/veh)	LOS
Jefferson Street & Parking Garage Driveway (Signalized)								
EB Approach	21.5	C	12.5	B	1.6	A	1.2	A
WB Approach	19.5	B	18.1	B	17.9	B	20.2	C
NB Approach	7.4	A	13.5	B	16.1	B	31.6	C
SB Approach	7.1	A	14.6	B	4.5	A	10.2	B
Intersection	11.5	B	15.2	B	13.6	B	22.0	C
Jefferson Street & Huron Street/Casino Lobby Driveway (Unsignalized)								
NB Left	7.9	A	8.2	A	8.3	A	8.6	A
EB Approach	15.8	C	24.1	C	31.6	D	60.6	F
SB Left	8.7	A	9.4	A	10.4	B	11.9	B
Jefferson Street & Erie Street/Hotel Lobby Driveway (Unsignalized)								
NB Left	7.9	A	8.3	A	8.3	A	8.5	A
EB Approach	16.1	C	26.9	D	47.1	E	>100	F
WB Right	12.6	B	15.6	C	21.3	C	39.5	E
SB Left	8.6	A	9.2	A	10.1	B	11.6	B
Jefferson Street & Desplaines Street/South Parking Lot Driveway (Unsignalized)								
NB Left/Thru	7.8	A	8.2	A	8.2	A	8.4	A
NB Thru/Right	0.0	A	0.0	A	0.0	A	0.0	A
EB Approach	19.7	C	33.4	D	41.1	E	88.5	F
WB Approach	19.2	C	30.1	D	32.6	D	64.3	F
SB Left/Thru	8.7	A	9.3	A	9.9	A	11.0	B
SB Thru/Right	0.0	A	0.0	A	0.0	A	0.0	A

Based on the capacity analysis results, all movements operate at LOS C or better during the weekday am commuter peak hour. During the weekday pm commuter peak hour, all movements operate at LOS D or better with the exception of the eastbound approach at Jefferson Street and Desplaines Street which is projected to operate at LOS E. Higher delays are observed during the Friday casino and Saturday casino peak hours with the eastbound approaches of Huron Street, Erie Street, and Desplaines Street operating at LOS F. All other movements operate at LOS D or better for the Friday and Saturday casino peak hours.

In the event that any intersections or movements operate with high delays, it is recommended that traffic control aides may be needed to improve traffic flow during those times. Additional details regarding traffic control aides is provided in a later section of this Technical Memorandum.



Queue Analysis – 2032 PD 1426 Full Build Out

A queue length analysis was conducted at each of the study area intersections for the full PD 1426 build out. A summary of this analysis is provided in Table 7. This data is comprised of the 95th percentile queue output from the Synchro analysis.

Table 7 – Full PD 1426 Build Out: 95th Percentile Queue Lengths

Intersection	Peak Period	95th Percentile Queue (feet)							
		Eastbound		Westbound		Northbound		Southbound	
		Left	Right	Left	Right	Left	Right	Left	Right
Jefferson Street and Chicago Avenue	Weekday Commuter AM Peak	30	5	111	0	135	158	62	-
	Weekday Commuter PM Peak	62	120	88	12	157	390	81	-
	Friday Evening Casino Peak	8	109	163	0	207	374	25	-
	Saturday Evening Casino Peak	6	96	213	0	265	234	25	-
Storage Length (ft)		60	55	95	55	190	190	50	-
Taper Length (ft)		50	55	90	55	75	75	50	-
Jefferson Street & Parking Garage Driveway (Signalized)	Weekday Commuter AM Peak	-	-	130	52	18	34	51	-
	Weekday Commuter PM Peak	-	-	195	53	26	81	88	-
	Friday Evening Casino Peak	-	-	205	146	18	70	21	-
	Saturday Evening Casino Peak	-	-	223	268	22	162	58	-
Storage Length (ft)		-	-	-	-	60	100	200	-
Taper Length (ft)		-	-	-	-	55	50	75	-
Jefferson Street & Huron Street/Casino Lobby Driveway (Unsignalized)	Weekday Commuter AM Peak	8	-	-	-	0	-	3	-
	Weekday Commuter PM Peak	15	-	-	-	0	-	3	-
	Friday Evening Casino Peak	13	-	-	-	0	-	5	-
	Saturday Evening Casino Peak	28	-	-	-	0	-	10	-
Storage Length (ft)		-	-	-	-	50	50	100	-
Taper Length (ft)		-	-	-	-	50	50	75	-
Jefferson Street & Erie Street/Hotel Lobby Driveway (Unsignalized)	Weekday Commuter AM Peak	8	-	10	0	-	-	3	-
	Weekday Commuter PM Peak	15	-	23	3	-	-	3	-
	Friday Evening Casino Peak	20	-	40	0	-	-	5	-
	Saturday Evening Casino Peak	68	-	90	3	-	-	10	-
Storage Length (ft)		-	-	-	-	50	-	100	-
Taper Length (ft)		-	-	-	-	50	-	50	-
Jefferson Street & Desplaines Street/South Parking Lot Driveway	Weekday Commuter AM Peak	10	-	23	-	-	-	-	-
	Weekday Commuter PM Peak	30	-	35	-	-	-	-	-
	Friday Evening Casino Peak	28	-	23	-	-	-	-	-
	Saturday Evening Casino Peak	58	-	45	-	-	-	-	-
Storage Length (ft)		-	-	-	-	-	-	-	-
Taper Length (ft)		-	-	-	-	-	-	-	-
Jefferson Street and Grand Avenue	Weekday Commuter AM Peak	72	-	-	-	-	-	131	37
	Weekday Commuter PM Peak	159	-	-	-	-	-	163	36
	Friday Evening Casino Peak	165	-	-	-	-	-	183	31
	Saturday Evening Casino Peak	228	-	-	-	-	-	205	26
Storage Length (ft)		100	-	-	-	-	-	100	100
Taper Length (ft)		100	-	-	-	-	-	180	180

Note: the northbound approach at Desplaines Street consists of one shared left turn/through lane and one shared through/right turn lane



The longest queues occur in the northbound direction at the Chicago Avenue and Jefferson Street intersection. It is anticipated that the queue lengths could back up into the parking garage intersection, blocking traffic from entering and exiting the driveway. It is recommended that traffic control aides be present to prevent northbound vehicles from blocking the driveway to maximize traffic flow through this area. Additionally, this intersection should be synchronized with the Parking Garage Driveway to maximize efficiency between intersections. Shifting additional green time from Chicago Avenue to Jefferson Street would likely decrease the 95th percentile queue lengths along Jefferson Street. It is recommended that this area be monitored in the future to maximize the efficiency of Chicago Avenue and Jefferson Street.

The westbound right turn queue at the Jefferson Street and Garage Driveway intersection during the Saturday evening casino peak hour is 268 feet, or approximately eleven vehicles. While the internal design and operations of the parking garage are still being finalized, particularly the locations of the gates for exiting vehicles, it is recommended that the exit gates be located more than 300 feet from this intersection to maximize the queue length such that queues from this intersection do not back up through the gates. If this does occur, it is recommended that traffic control aides provided by Bally's may be needed to improve traffic flow.

The eastbound left turn queue at Jefferson Street and Grand Avenue may be longer than the left turn storage length for some of the peak hours. This is due to the offset intersection for the north/south movements to align with the existing public right of way to the south and not the existing private residential driveway. This offset results in the need for a split phase due to the overlapping northbound and southbound left turn movements. When the south roadway is constructed, it is anticipated that a traditional signal timing plan can be implemented which would allow more green time for the westbound and southbound left turn movements.

The analysis results indicate that there are no other significant queueing issues at the other study area intersections.

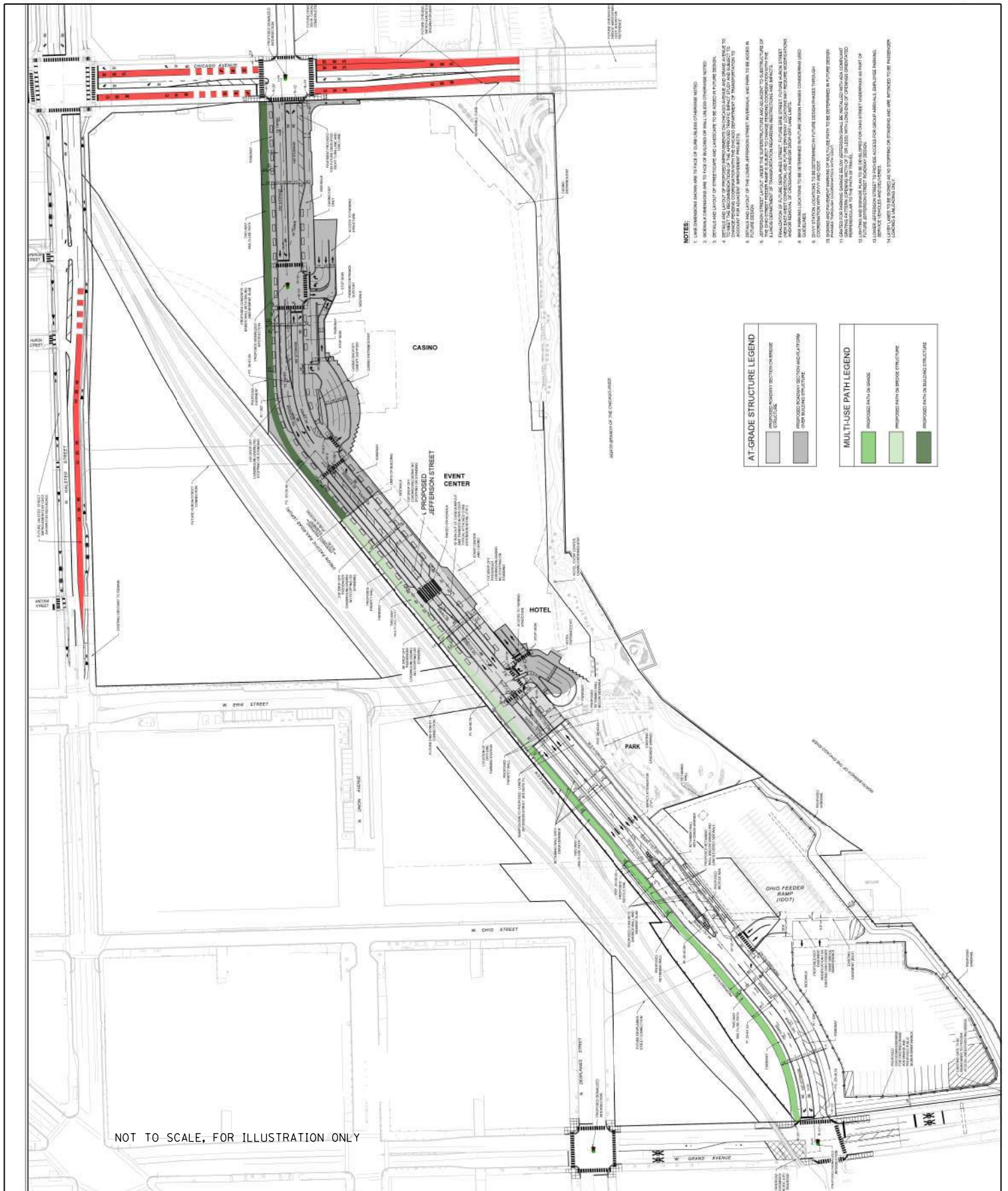
Figure 9 illustrates the proposed lane configuration at the study area intersections for the full PD 1426 Build Out scenario.

Traffic Control Aides

In order to provide safe and efficient access for entertainment district patrons as well as to limit any roadway impacts to the local businesses and neighborhood, it is recommended that Traffic Control Aides (TCA's) be present as needed along Jefferson Street at the Parking Garage Driveway/Private Driveway and at the Casino Lobby Driveway/Huron Street during the peak casino periods on Friday and Saturday evenings. Using TCAs at these and other locations around the entertainment district will be re-evaluated periodically over time and adjusted accordingly at the discretion of Bally's to verify that adequate TCA coverage is provided to efficiently serve all entertainment district patrons and adjacent modes of transportation. It is anticipated that private TCA's will be utilized along Jefferson Street adjacent to the entertainment district.



The roles of the TCA's are to keep traffic flowing around the site, keep the intersections clear, limit double parking along the roadways, provide opportunities for minor approaches to enter and exit without causing backups and delay, and limit potential traffic queues. They will be able to limit the queue lengths and delay times for the impacted movements at both driveways by temporarily stopping traffic on Jefferson Street to allow the outbound movements from the driveway. This will minimize queue lengths backing up into the valet operation and hotel rideshare pick up/drop off area. Additional roles will be to enforce the no parking rules, keep rideshare vehicles and taxis from backing up onto the street, enforce no double parking, move pedestrians across the streets, and minimize dwell time and delays for buses, valet, and rideshare vehicles. Since the TCA's will likely be private, ticketing vehicles may not be an option, however towing vehicles along Jefferson Street will be an option.



- NOTE:**
1. THE PROPOSED DEVELOPMENT IS SUBJECT TO ALL CITY AND COUNTY ORDINANCES.
 2. THE PROPOSED DEVELOPMENT IS SUBJECT TO ALL CITY AND COUNTY ORDINANCES.
 3. DETAILS AND LAYOUT OF PROPOSED IMPROVEMENTS ON CHICAGO AVENUE AND GRAND AVENUE TO BE DETERMINED IN A SEPARATE STUDY.
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 20. THE PROPOSED DEVELOPMENT IS SUBJECT TO ALL CITY AND COUNTY ORDINANCES.

AT-GRADE STRUCTURE LEGEND	
[Light Gray Box]	PROPOSED MULTI-USE STRUCTURE
[Medium Gray Box]	PROPOSED MULTI-USE STRUCTURE
[Dark Gray Box]	PROPOSED MULTI-USE STRUCTURE

MULTI-USE PATH LEGEND	
[Light Green Box]	PROPOSED PATH OR BRIDGE
[Medium Green Box]	PROPOSED PATH OR BRIDGE STRUCTURE
[Dark Green Box]	PROPOSED PATH OR BRIDGE STRUCTURE

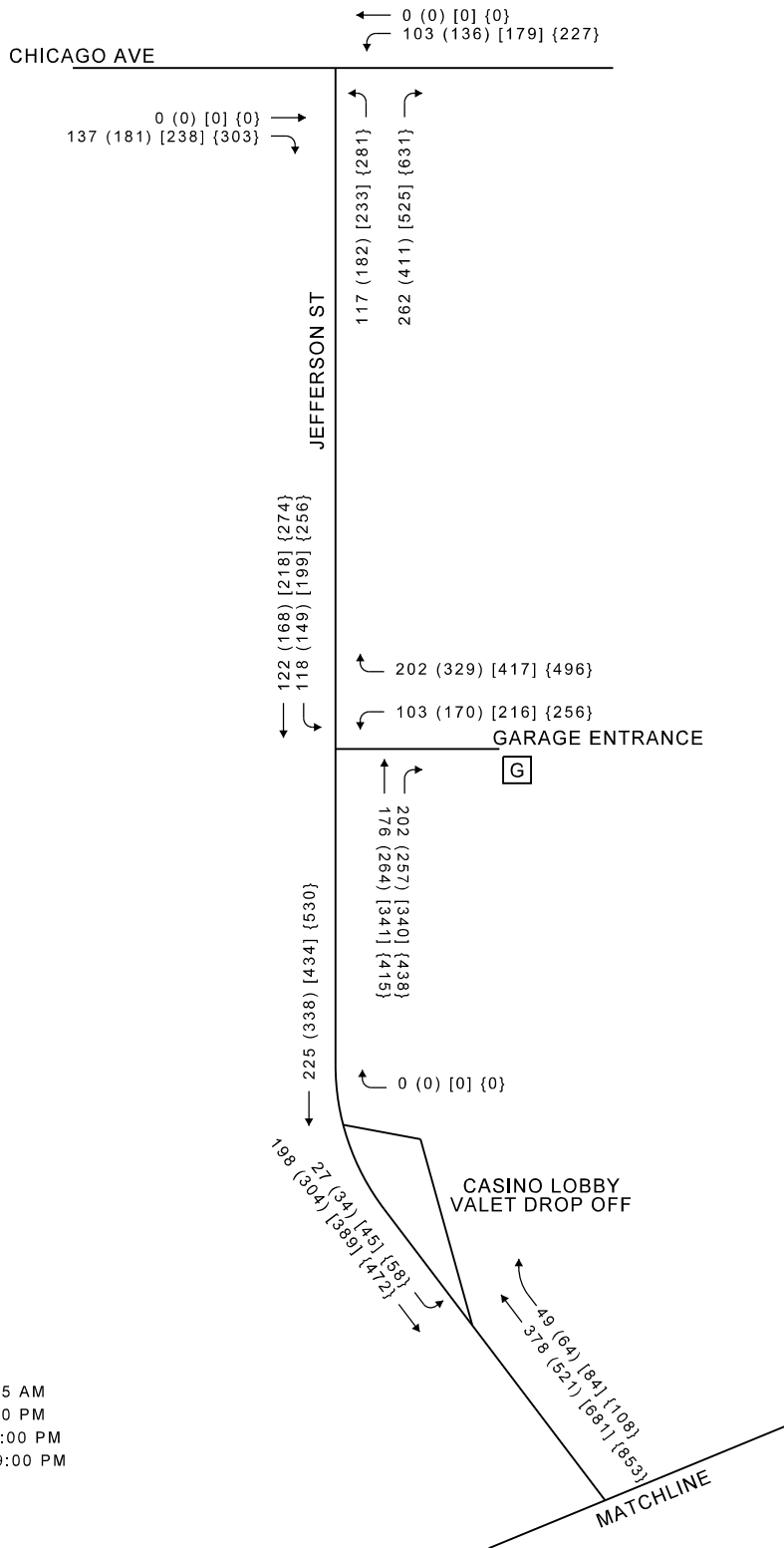
BALLY'S CHICAGO CASINO

**FIGURE 1
ENTERTAINMENT DISTRICT
CONCEPTUAL SITE PLAN**

CHICAGO

ILLINOIS





LEGEND

- ## - AM PEAK HOUR
- (##) - PM PEAK HOUR
- [##] - FRI CASINO PEAK
- {##} - SAT CASINO PEAK

AM PEAK HOUR: 7:45 AM - 8:45 AM
 PM PEAK HOUR: 5:00 PM - 6:00 PM
 FRI CASINO PEAK: 8:00 PM - 9:00 PM
 SAT CASINO PEAK: 8:00 PM - 9:00 PM

G - GARAGE

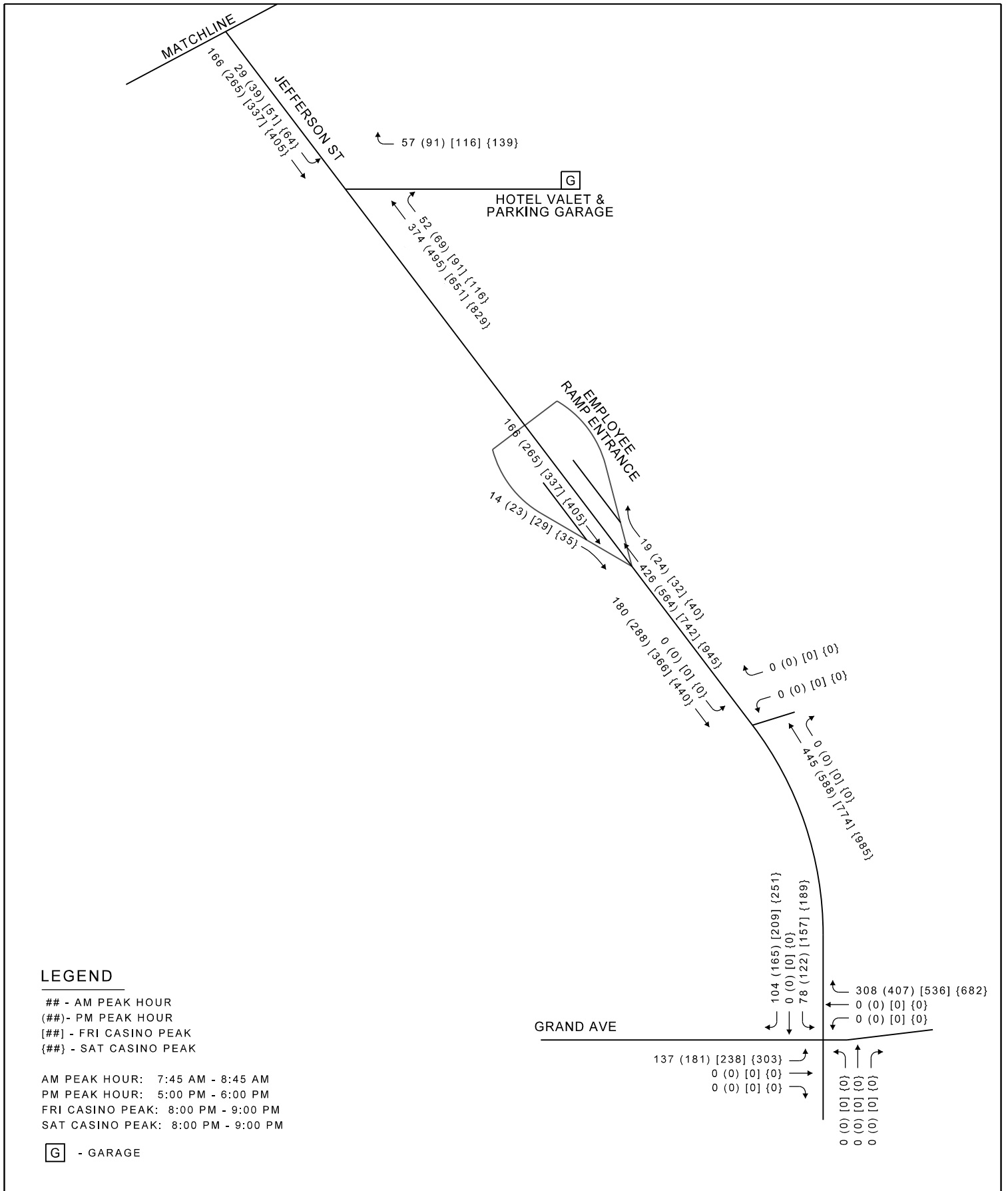
BALLY'S CHICAGO CASINO

**FIGURE 2
 ENTERTAINMENT DISTRICT
 SITE TRAFFIC VOLUMES**

CHICAGO

ILLINOIS





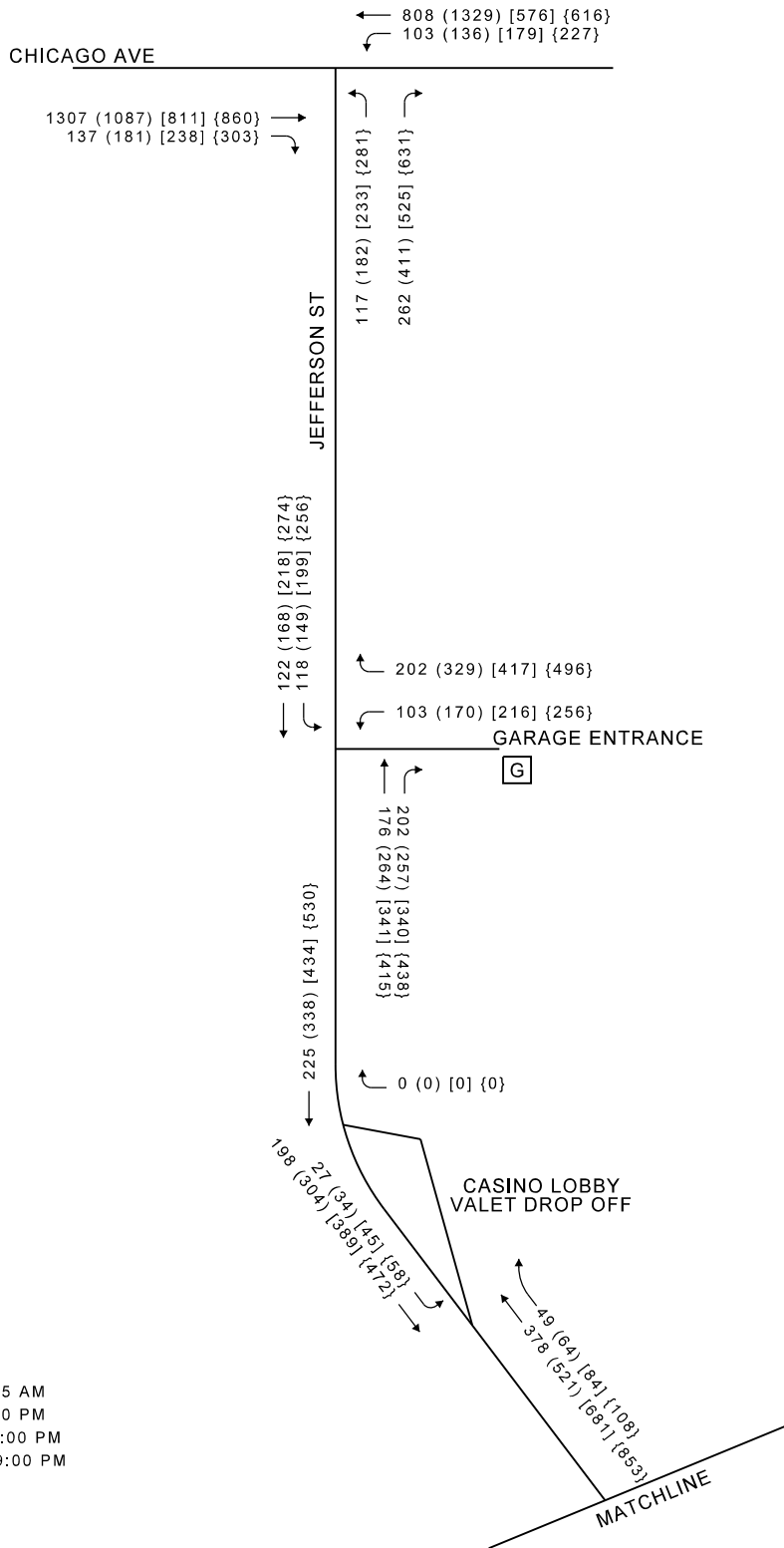
BALLY'S CHICAGO CASINO

**FIGURE 2
 ENTERTAINMENT DISTRICT
 SITE TRAFFIC VOLUMES**

CHICAGO

ILLINOIS





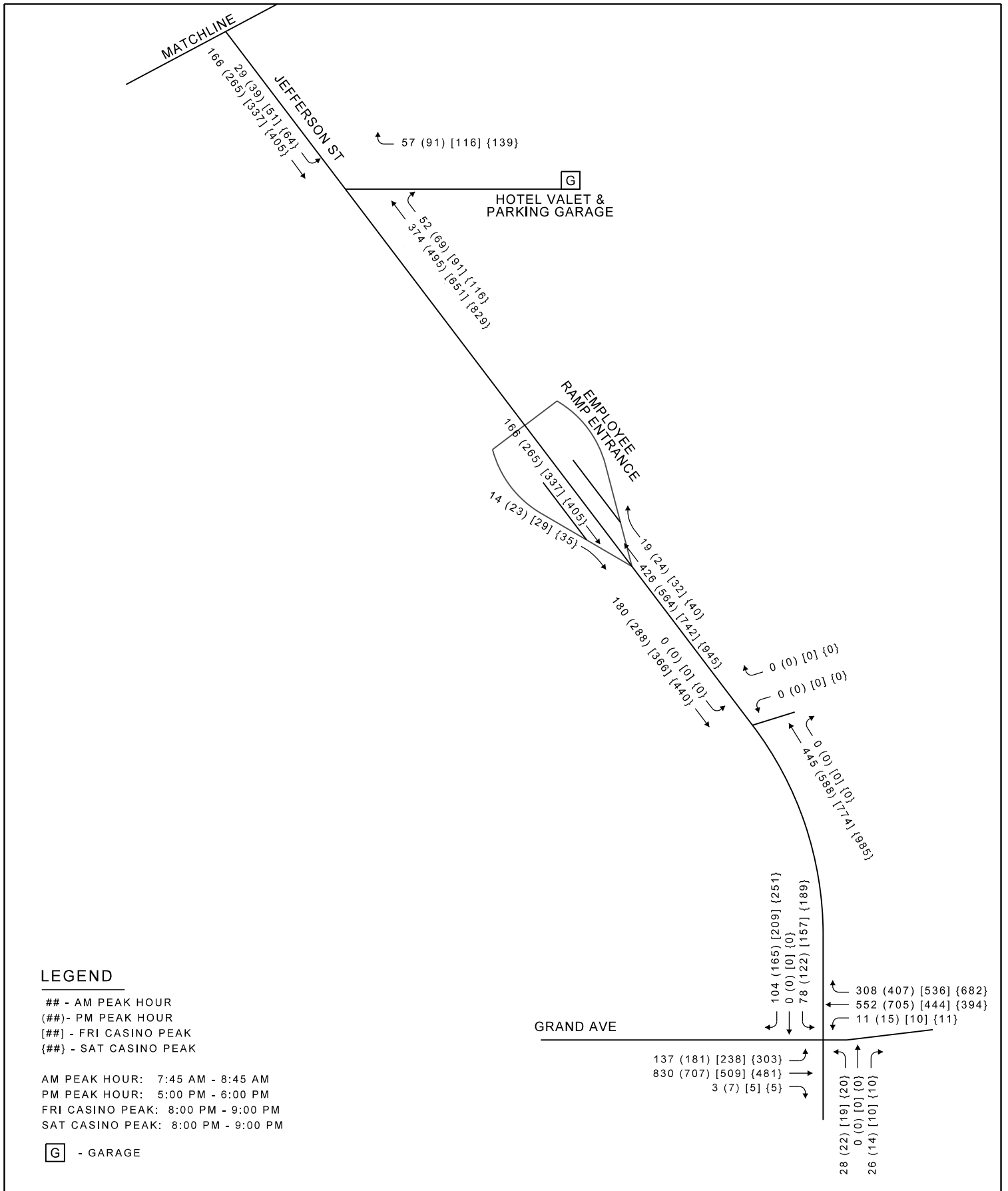
BALLY'S CHICAGO CASINO

**FIGURE 3
FUTURE WITH ENTERTAINMENT
DISTRICT TRAFFIC VOLUMES**

CHICAGO

ILLINOIS





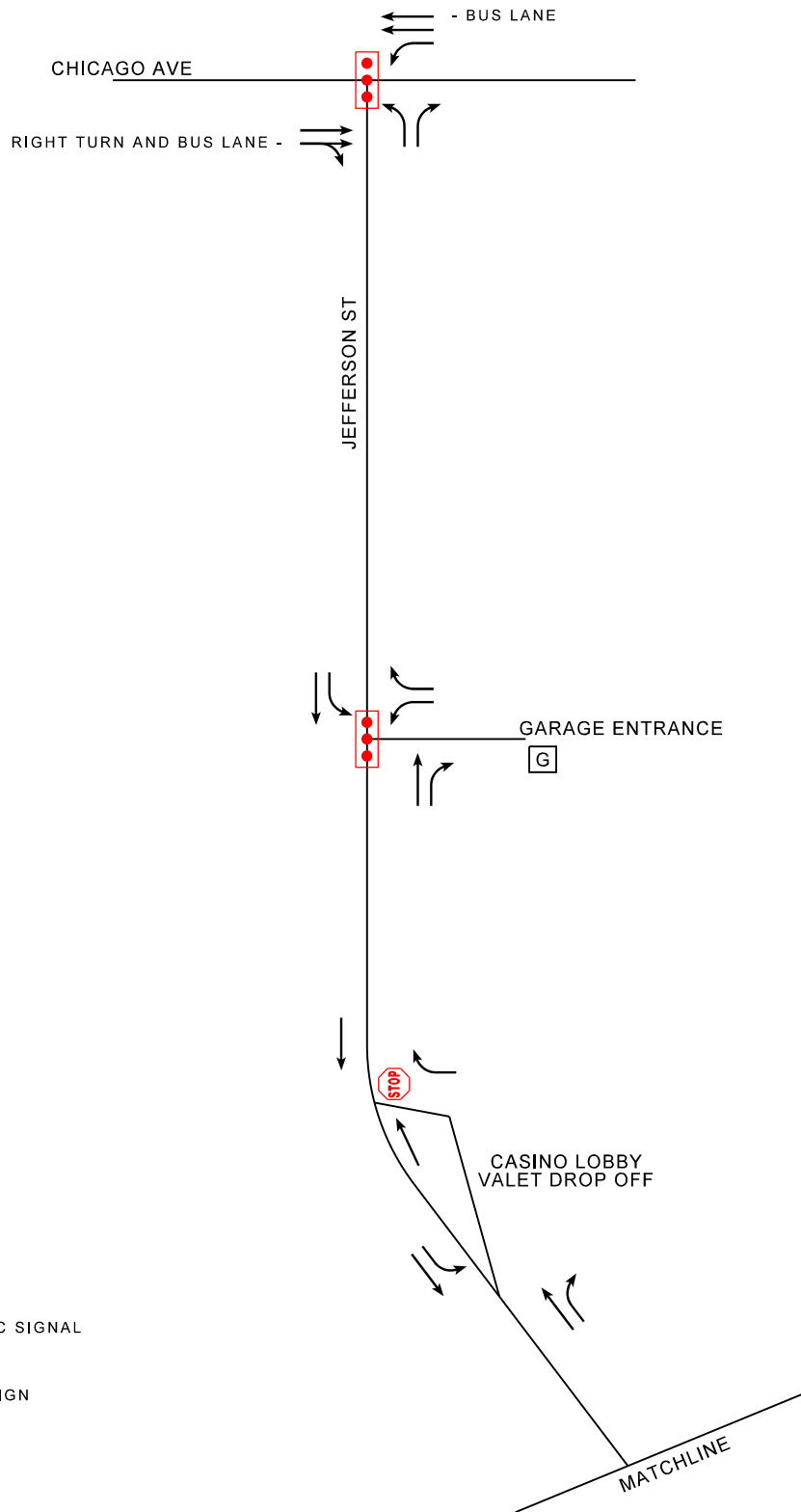
BALLY'S CHICAGO CASINO

**FIGURE 3
 FUTURE WITH ENTERTAINMENT
 DISTRICT TRAFFIC VOLUMES**




CHICAGO

ILLINOIS





LEGEND

-  - PROPOSED TRAFFIC SIGNAL
-  - PROPOSED STOP SIGN
-  - GARAGE

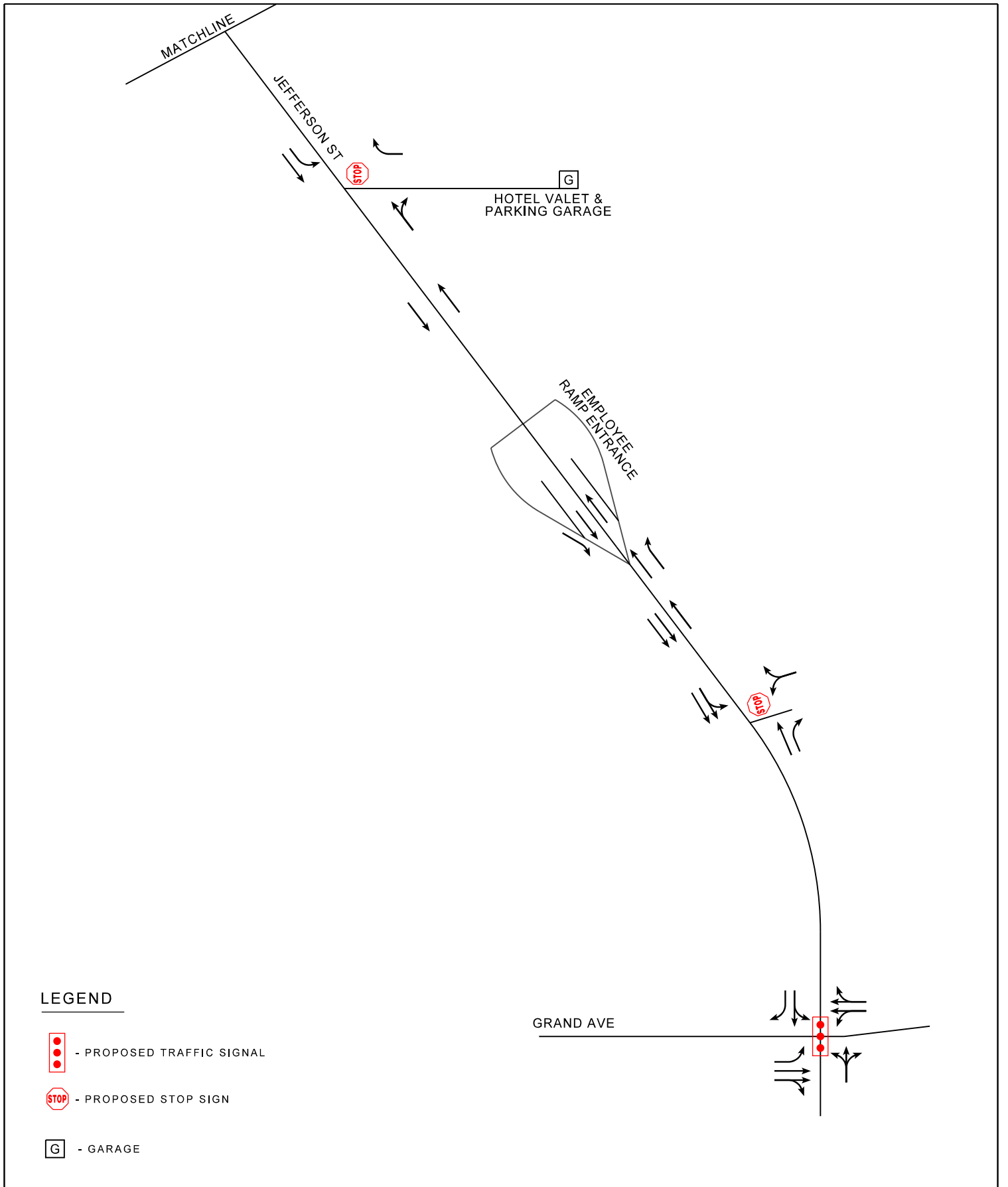
BALLY'S CHICAGO CASINO

**FIGURE 4
FUTURE WITH ENTERTAINMENT
DISTRICT LANE CONFIGURATION**




CHICAGO

ILLINOIS





LEGEND

-  - PROPOSED TRAFFIC SIGNAL
-  - PROPOSED STOP SIGN
-  - GARAGE

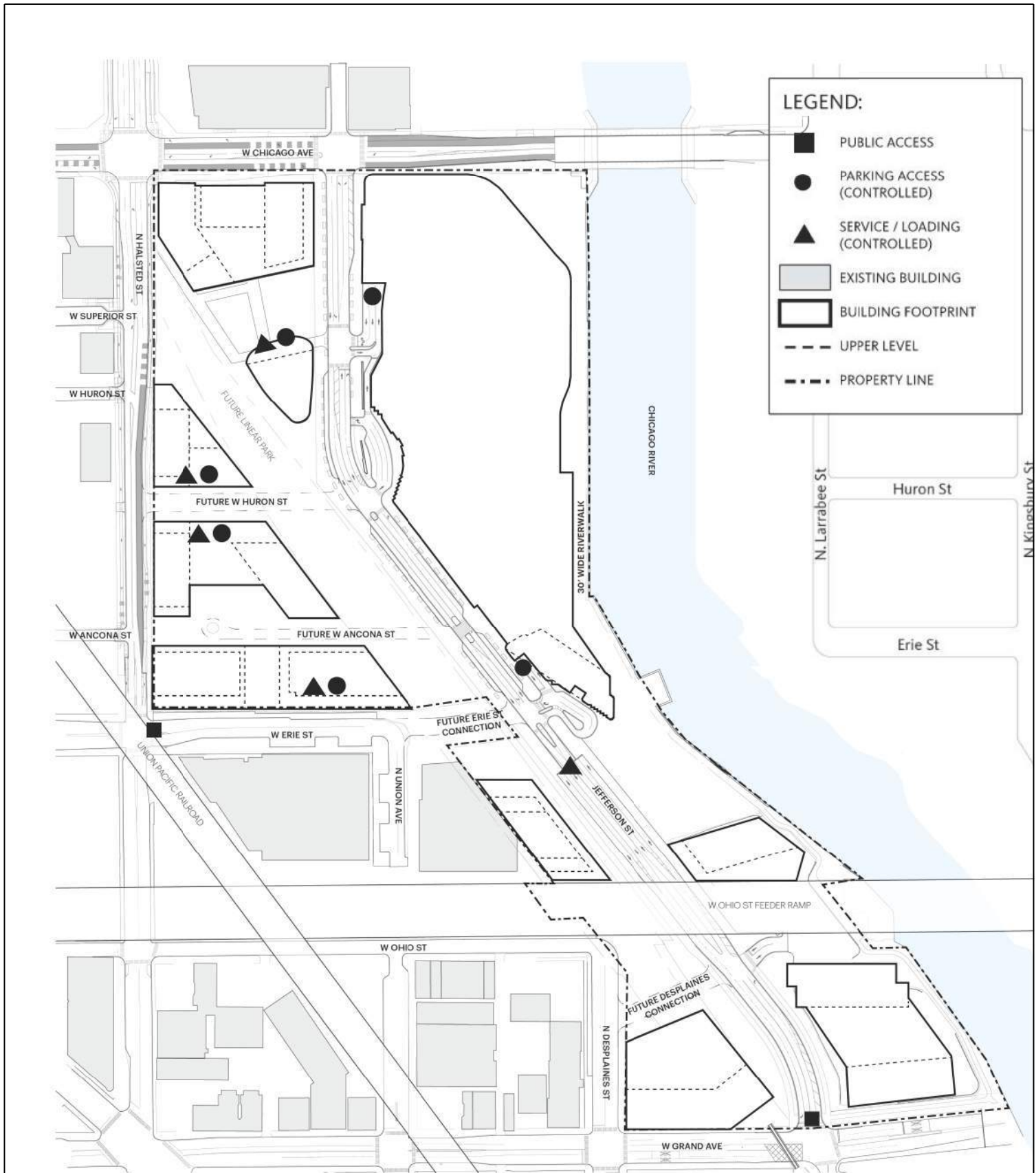
BALLY'S CHICAGO CASINO

**FIGURE 4
FUTURE WITH ENTERTAINMENT
DISTRICT LANE CONFIGURATION**

CHICAGO

ILLINOIS





NOT TO SCALE, FOR ILLUSTRATION ONLY

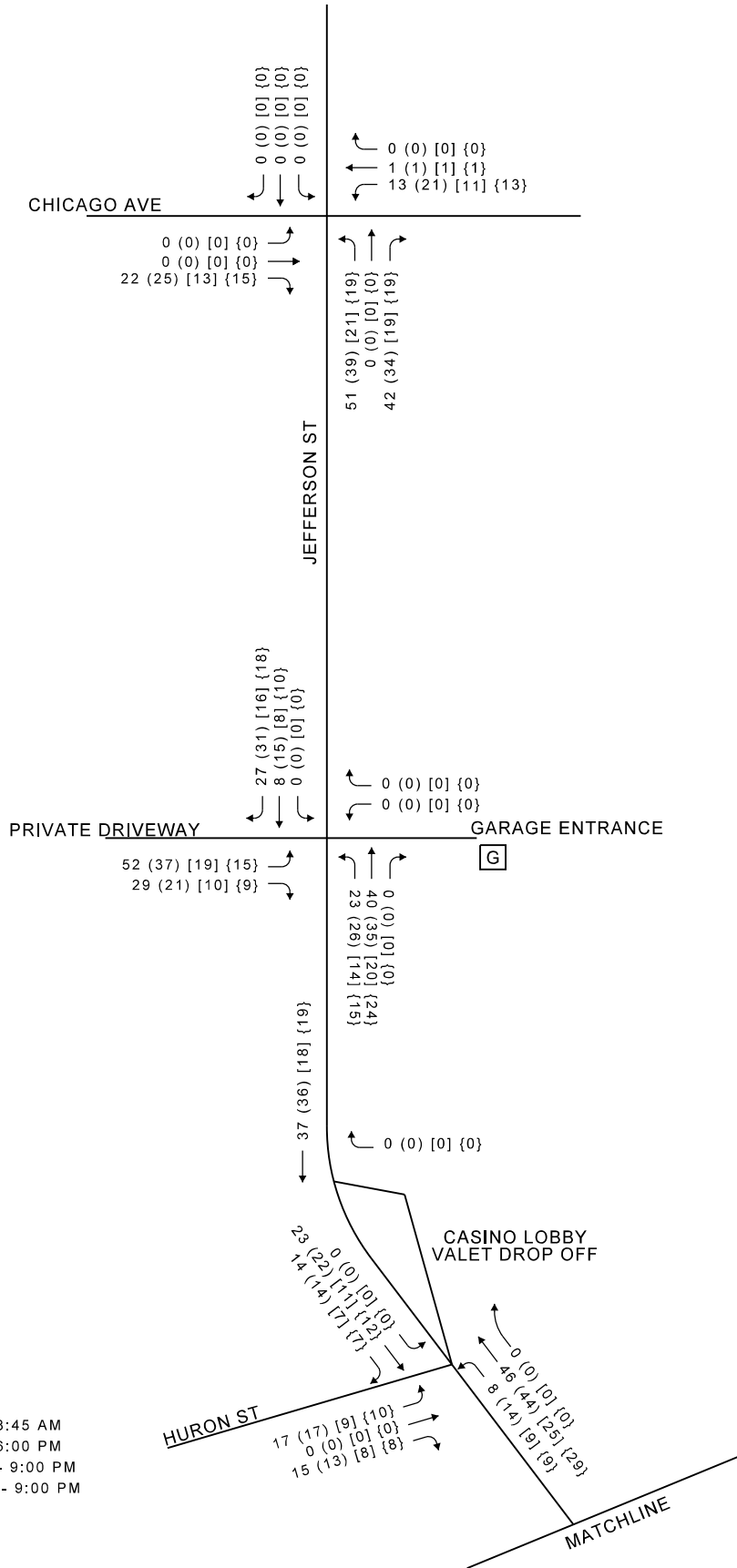
BALLY'S CHICAGO CASINO

FIGURE 5 FULL PD 1426 CONCEPTUAL SITE PLAN

CHICAGO

ILLINOIS





LEGEND

- ## - AM PEAK HOUR
- (##) - PM PEAK HOUR
- [##] - FRI CASINO PEAK
- {##} - SAT CASINO PEAK

AM PEAK HOUR: 7:45 AM - 8:45 AM
 PM PEAK HOUR: 5:00 PM - 6:00 PM
 FRI CASINO PEAK: 8:00 PM - 9:00 PM
 SAT CASINO PEAK: 8:00 PM - 9:00 PM

G - GARAGE

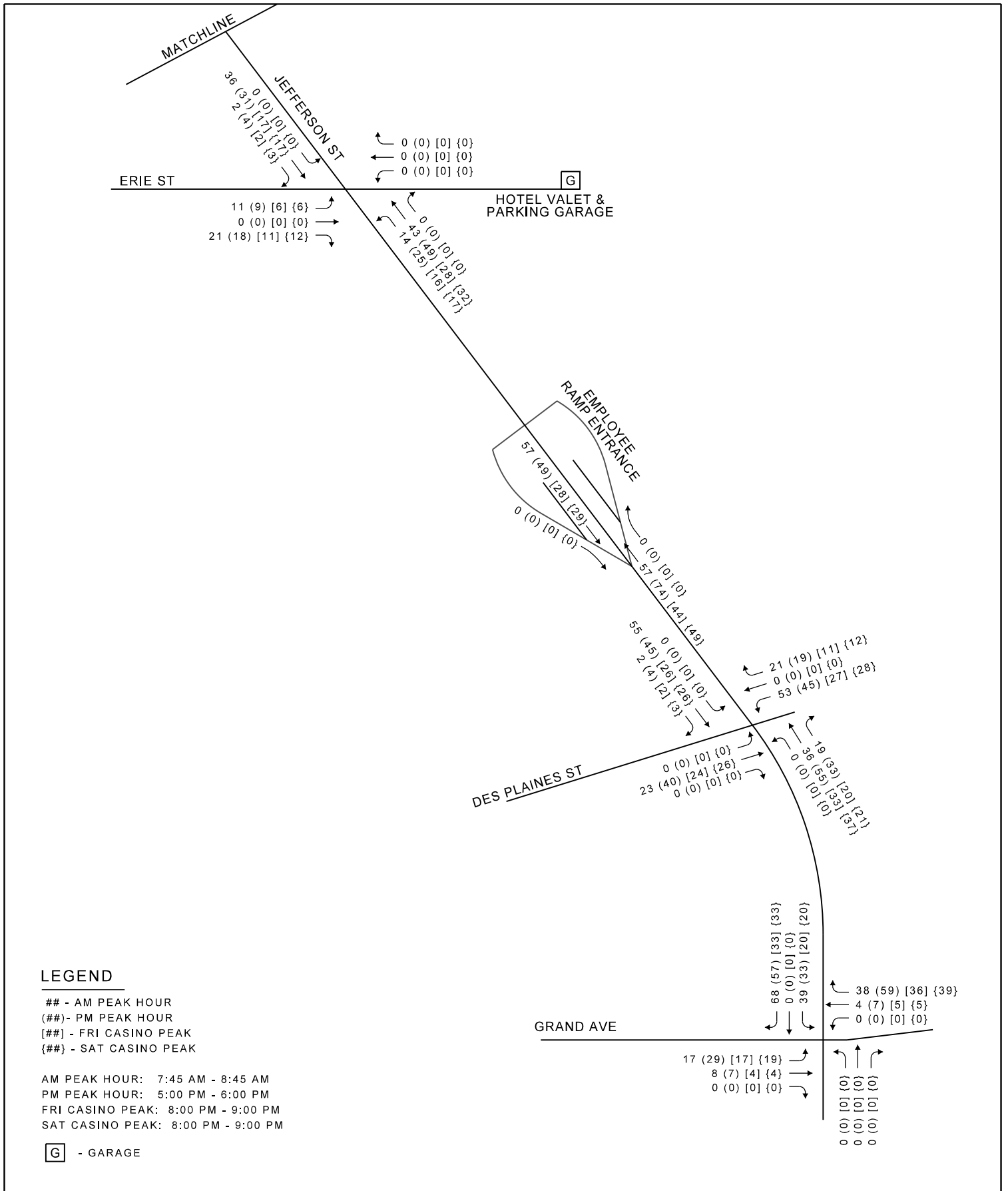
BALLY'S CHICAGO CASINO

**FIGURE 6
 REMAINING PD 1426 PARCELS
 SITE TRAFFIC VOLUMES**

CHICAGO

ILLINOIS





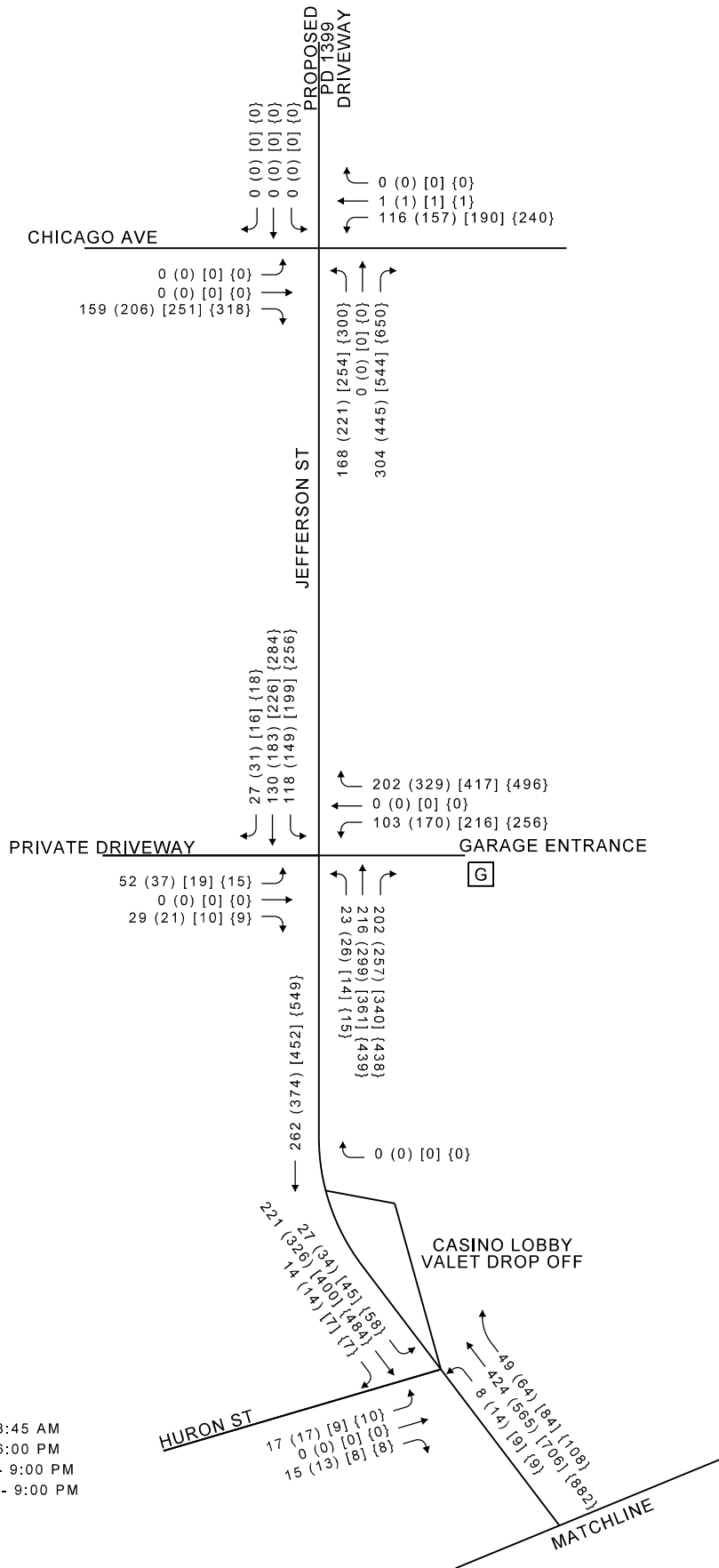
BALLY'S CHICAGO CASINO

**FIGURE 6
 REMAINING PD 1426 PARCELS
 SITE TRAFFIC VOLUMES**

CHICAGO

ILLINOIS





LEGEND

- ## - AM PEAK HOUR
- (##) - PM PEAK HOUR
- [##] - FRI CASINO PEAK
- {##} - SAT CASINO PEAK

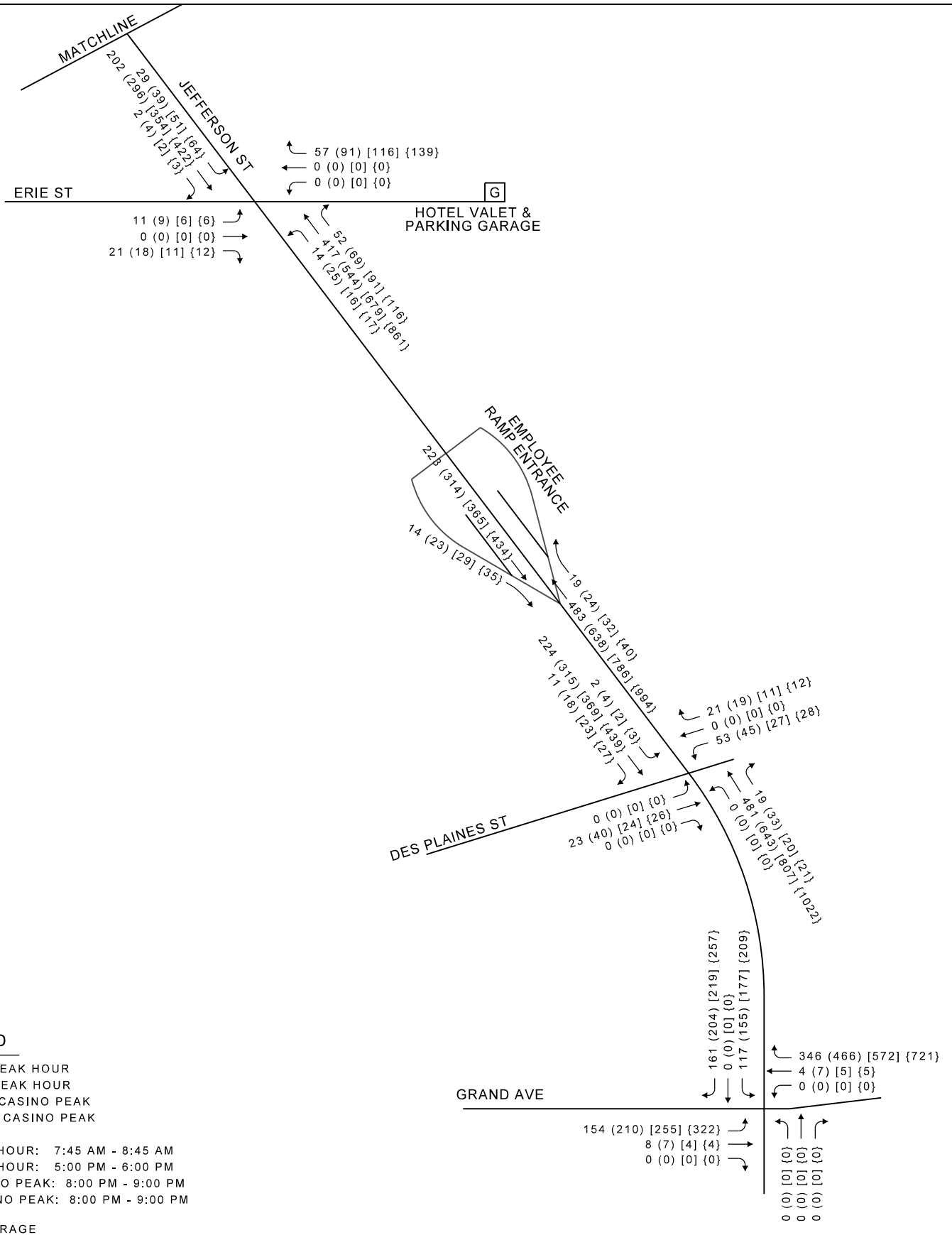
AM PEAK HOUR: 7:45 AM - 8:45 AM
 PM PEAK HOUR: 5:00 PM - 6:00 PM
 FRI CASINO PEAK: 8:00 PM - 9:00 PM
 SAT CASINO PEAK: 8:00 PM - 9:00 PM

G - GARAGE

BALLY'S CHICAGO CASINO

**FIGURE 7
 FULL PD1426 SITE
 TRAFFIC VOLUMES**

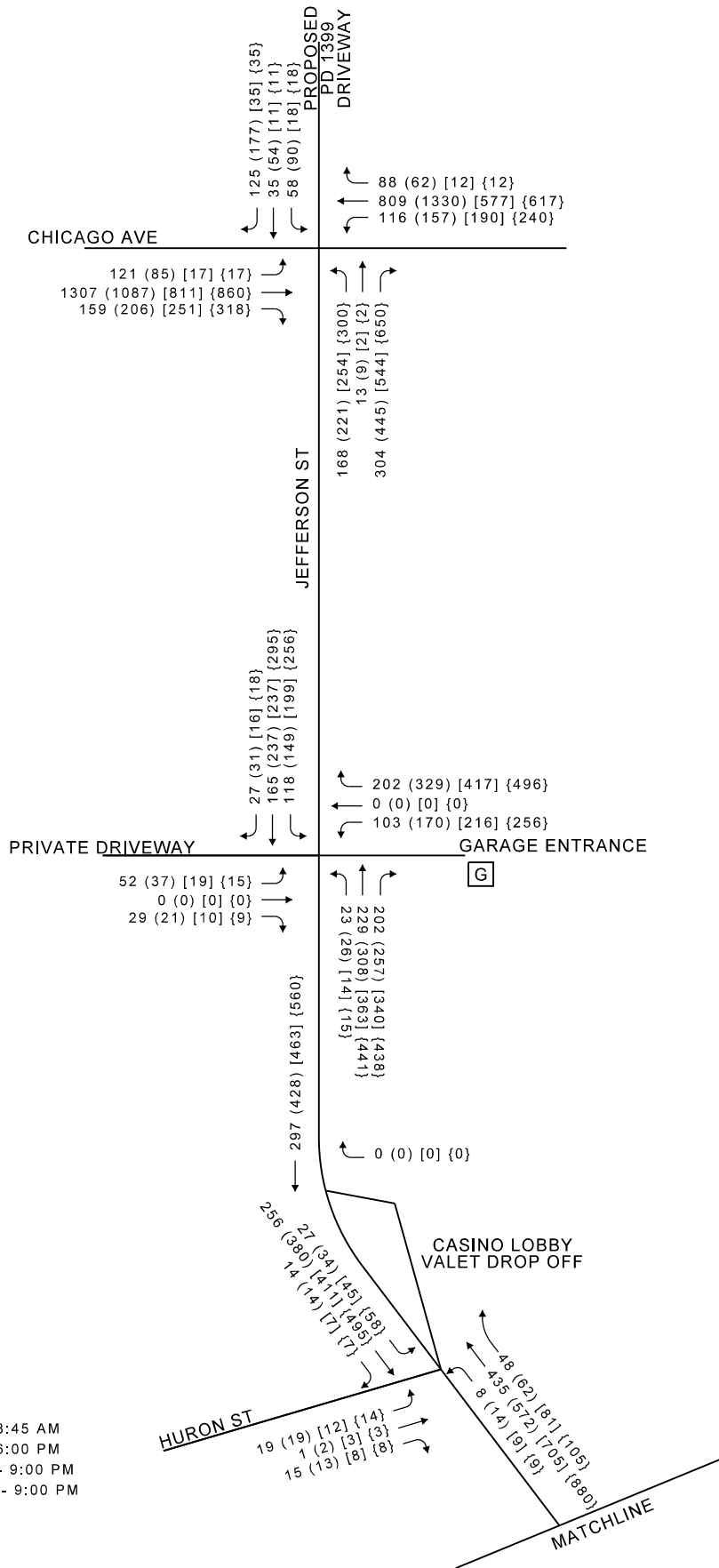




BALLY'S CHICAGO CASINO

**FIGURE 7
 FULL PD1426 SITE
 TRAFFIC VOLUMES**





LEGEND

- ## - AM PEAK HOUR
- (##) - PM PEAK HOUR
- [##] - FRI CASINO PEAK
- {##} - SAT CASINO PEAK

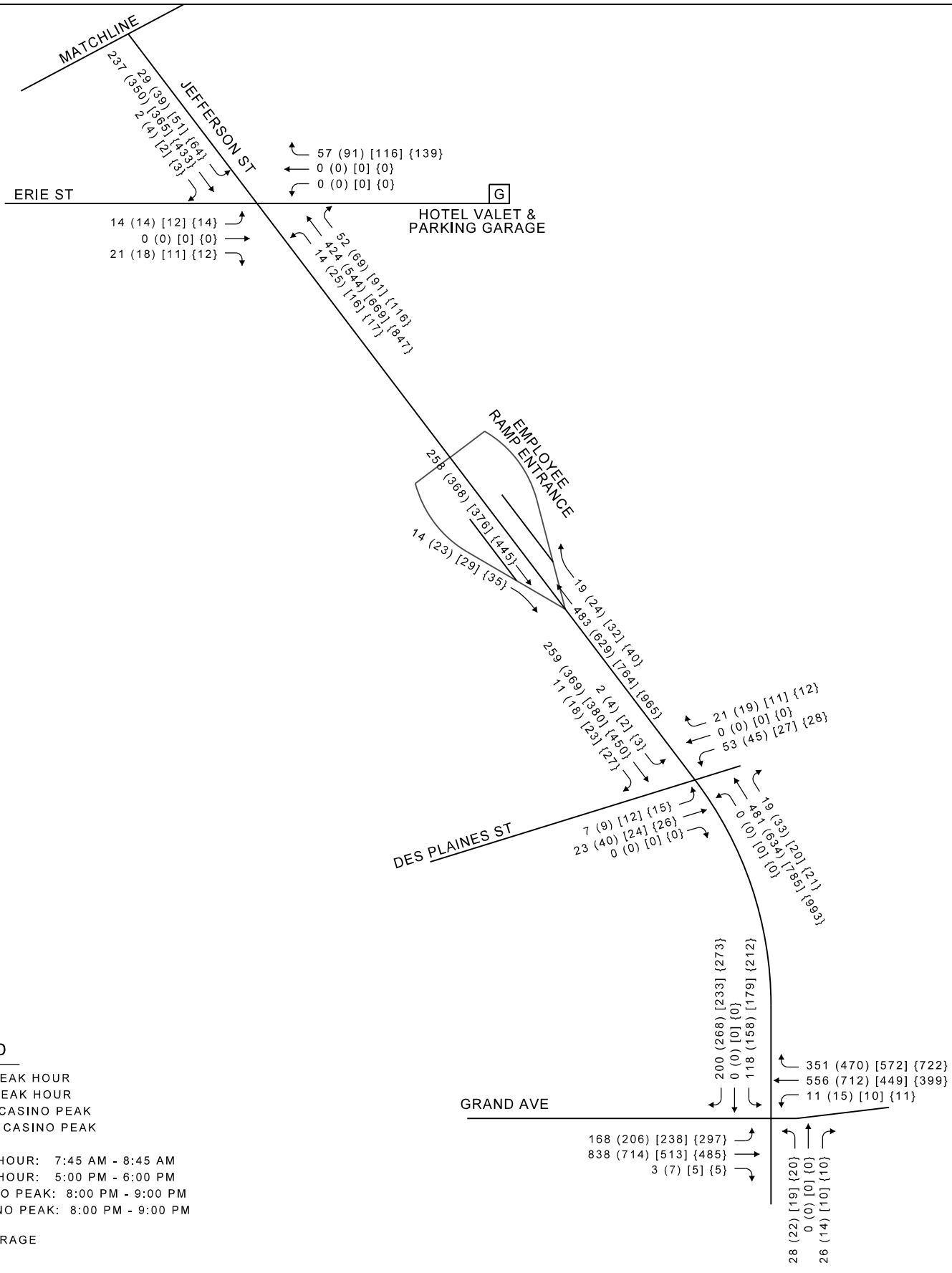
AM PEAK HOUR: 7:45 AM - 8:45 AM
 PM PEAK HOUR: 5:00 PM - 6:00 PM
 FRI CASINO PEAK: 8:00 PM - 9:00 PM
 SAT CASINO PEAK: 8:00 PM - 9:00 PM

G - GARAGE

BALLY'S CHICAGO CASINO

**FIGURE 8
 FUTURE WITH PD1426
 TRAFFIC VOLUMES**





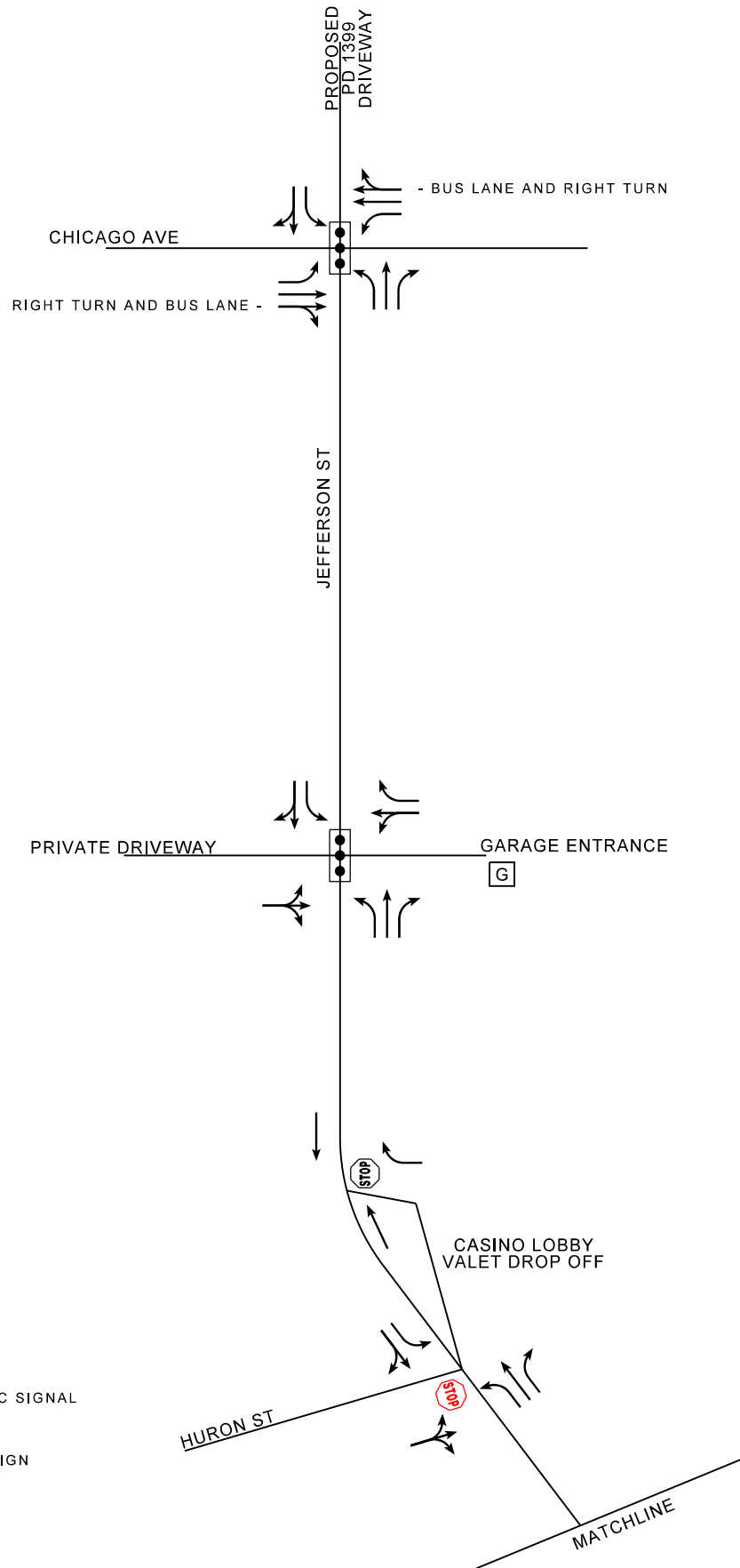
BALLY'S CHICAGO CASINO

CHICAGO




**FIGURE 8
 FUTURE WITH PD1426
 TRAFFIC VOLUMES**

ILLINOIS





LEGEND

-  - PROPOSED TRAFFIC SIGNAL
-  - PROPOSED STOP SIGN
-  - GARAGE

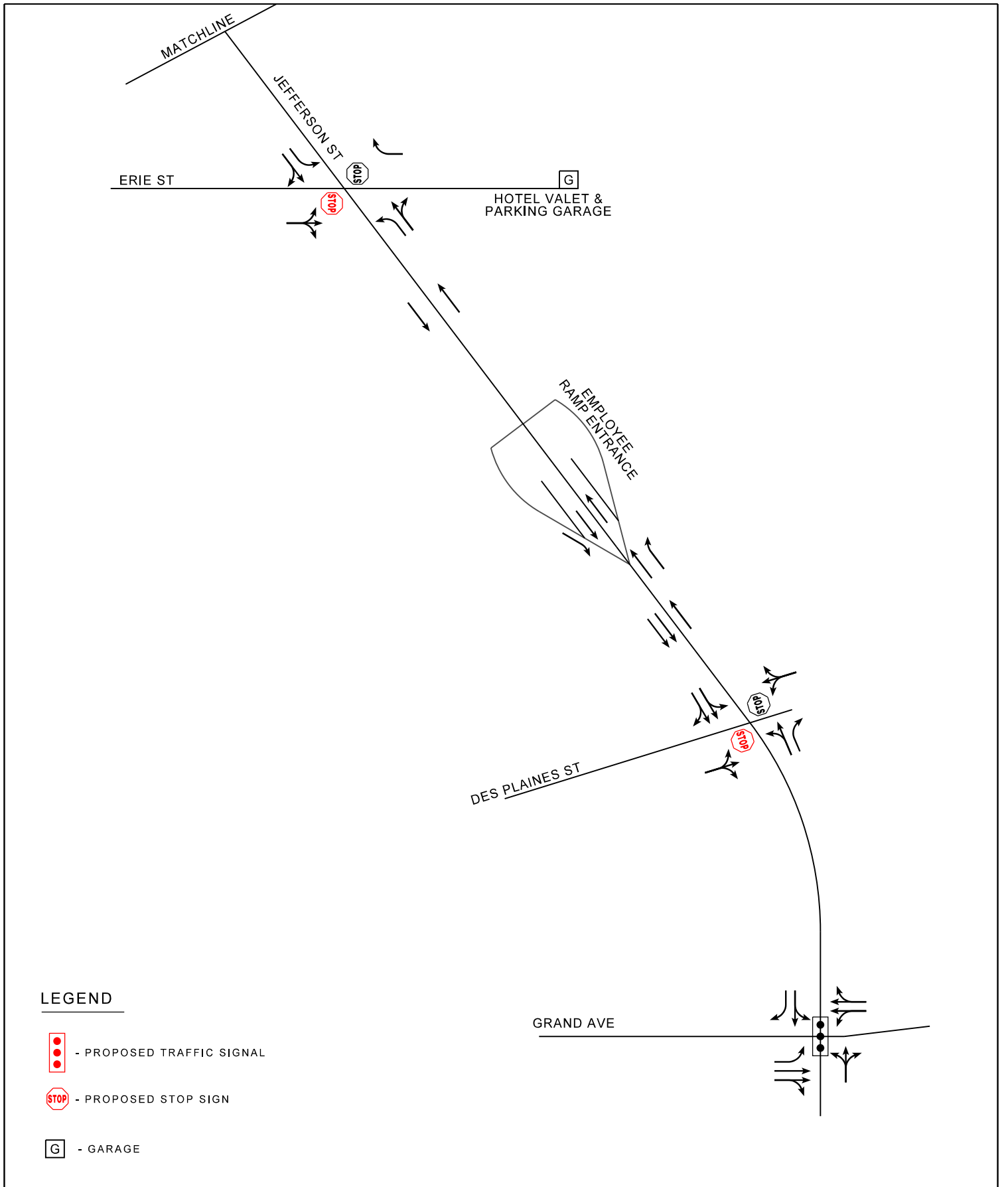
BALLY'S CHICAGO CASINO

**FIGURE 9
FUTURE WITH PD1426
LANE CONFIGURATION**




CHICAGO

ILLINOIS





LEGEND

-  - PROPOSED TRAFFIC SIGNAL
-  - PROPOSED STOP SIGN
-  - GARAGE

BALLY'S CHICAGO CASINO

**FIGURE 9
FUTURE WITH PD1426
LANE CONFIGURATION**

CHICAGO

ILLINOIS





Capacity Analysis Worksheets 2026 Entertainment District

	→	↘	↙	←	↖	↗
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↑	↗	↙	↑	↖	↗
Traffic Volume (vph)	1307	137	103	808	117	262
Future Volume (vph)	1307	137	103	808	117	262
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)		55	95		190	190
Storage Lanes		1	1		0	1
Taper Length (ft)			90		75	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor		0.94			0.93	0.97
Frt		0.850				0.850
Flt Protected			0.950		0.950	
Satd. Flow (prot)	1863	1583	1770	1863	1770	1583
Flt Permitted			0.056		0.950	
Satd. Flow (perm)	1863	1493	104	1863	1648	1530
Right Turn on Red		Yes				Yes
Satd. Flow (RTOR)		27				38
Link Speed (mph)	30			30	30	
Link Distance (ft)	390			893	363	
Travel Time (s)	8.9			20.3	8.3	
Confl. Peds. (#/hr)		25	25		25	25
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	1421	149	112	878	127	285
Shared Lane Traffic (%)						
Lane Group Flow (vph)	1421	149	112	878	127	285
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Left	Left	Right
Median Width(ft)	12			12	12	
Link Offset(ft)	0			0	0	
Crosswalk Width(ft)	16			16	16	
Two way Left Turn Lane						
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)		9	15		15	9
Number of Detectors	2	1	1	2	1	1
Detector Template	Thru	Right	Left	Thru	Left	Right
Leading Detector (ft)	100	20	20	100	20	20
Trailing Detector (ft)	0	0	0	0	0	0
Detector 1 Position(ft)	0	0	0	0	0	0
Detector 1 Size(ft)	6	20	20	6	20	20
Detector 1 Type	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex
Detector 1 Channel						
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 2 Position(ft)	94			94		
Detector 2 Size(ft)	6			6		
Detector 2 Type	Cl+Ex			Cl+Ex		
Detector 2 Channel						
Detector 2 Extend (s)	0.0			0.0		
Turn Type	NA	Perm	pm+pt	NA	Prot	pt+ov



Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Protected Phases	4		3	8	5	5 3
Permitted Phases		4	8		5	5 3
Detector Phase	4	4	3	8	5	5 3
Switch Phase						
Minimum Initial (s)	5.0	5.0	3.0	5.0	3.0	
Minimum Split (s)	58.0	58.0	8.0	58.0	8.0	
Total Split (s)	70.0	70.0	15.0	85.0	25.0	
Total Split (%)	63.6%	63.6%	13.6%	77.3%	22.7%	
Maximum Green (s)	65.0	65.0	12.0	80.0	20.0	
Yellow Time (s)	3.0	3.0	3.0	3.0	3.0	
All-Red Time (s)	2.0	2.0	0.0	2.0	2.0	
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	
Total Lost Time (s)	5.0	5.0	3.0	5.0	5.0	
Lead/Lag	Lag	Lag	Lead			
Lead-Lag Optimize?	Yes	Yes	Yes			
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	
Recall Mode	C-Max	C-Max	None	C-Max	Max	
Walk Time (s)	7.0	7.0		7.0		
Flash Dont Walk (s)	11.0	11.0		11.0		
Pedestrian Calls (#/hr)	0	0		0		
Act Effct Green (s)	68.7	68.7	82.0	80.0	20.0	31.3
Actuated g/C Ratio	0.62	0.62	0.75	0.73	0.18	0.28
v/c Ratio	1.22	0.16	0.55	0.65	0.40	0.60
Control Delay	117.2	4.2	47.1	10.1	40.5	32.0
Queue Delay	0.9	0.0	0.0	35.6	1.2	0.7
Total Delay	118.1	4.2	47.1	45.7	41.7	32.6
LOS	F	A	D	D	D	C
Approach Delay	107.3			45.9	35.4	
Approach LOS	F			D	D	
90th %ile Green (s)	65.0	65.0	12.0	80.0	20.0	
90th %ile Term Code	Coord	Coord	Max	Coord	MaxR	
70th %ile Green (s)	67.5	67.5	9.5	80.0	20.0	
70th %ile Term Code	Coord	Coord	Gap	Coord	MaxR	
50th %ile Green (s)	69.6	69.6	7.4	80.0	20.0	
50th %ile Term Code	Coord	Coord	Gap	Coord	MaxR	
30th %ile Green (s)	70.2	70.2	6.8	80.0	20.0	
30th %ile Term Code	Coord	Coord	Gap	Coord	MaxR	
10th %ile Green (s)	71.0	71.0	6.0	80.0	20.0	
10th %ile Term Code	Coord	Coord	Gap	Coord	MaxR	
Queue Length 50th (ft)	~1233	17	42	282	80	153
Queue Length 95th (ft)	m#767	m11	m92	296	144	243
Internal Link Dist (ft)	310			813	283	
Turn Bay Length (ft)		55	95		190	190
Base Capacity (vph)	1162	942	259	1354	321	529
Starvation Cap Reductn	200	0	0	220	0	66
Spillback Cap Reductn	0	0	0	527	74	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	1.48	0.16	0.43	1.06	0.51	0.62

Intersection Summary

Area Type: Other

Cycle Length: 110

Actuated Cycle Length: 110

Offset: 16 (15%), Referenced to phase 4:EBT and 8:WBTL, Start of Green

Natural Cycle: 90

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 1.22

Intersection Signal Delay: 76.9 Intersection LOS: E

Intersection Capacity Utilization 95.7% ICU Level of Service F

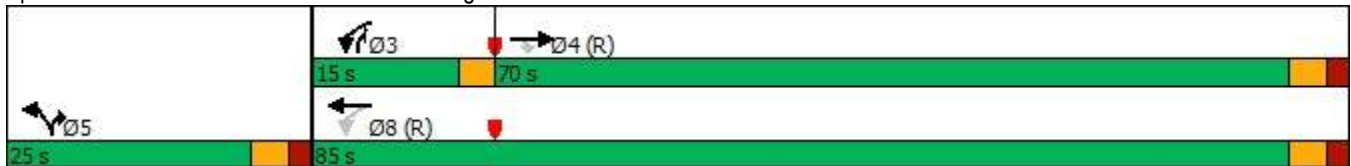
Analysis Period (min) 15

~ Volume exceeds capacity, queue is theoretically infinite.
 Queue shown is maximum after two cycles.

95th percentile volume exceeds capacity, queue may be longer.
 Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 204: Jefferson St & Chicago Ave



Bally's Chicago Casino
305: Canal St/Jefferson St & Grand Ave

2026 Future with Entertainment District
Timing Plan: AM Peak Hour



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	137	830	3	11	552	308	28	0	26	78	0	104
Future Volume (vph)	137	830	3	11	552	308	28	0	26	78	0	104
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	100		0	0		0	0		0	0		0
Storage Lanes	1		0	0		0	0		0	0		1
Taper Length (ft)	100			25			25			135		
Lane Util. Factor	1.00	0.95	0.95	0.95	0.95	0.95	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor		1.00			0.91			0.97				0.95
Fr _t					0.947			0.935				0.850
Fl _t Protected	0.950				0.999			0.975			0.950	
Satd. Flow (prot)	1770	3538	0	0	3036	0	0	1649	0	0	1770	1583
Fl _t Permitted	0.237				0.941			0.975			0.950	
Satd. Flow (perm)	441	3538	0	0	2859	0	0	1649	0	0	1770	1508
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)					110			109				113
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		375			838			269			430	
Travel Time (s)		8.5			19.0			6.1			9.8	
Confl. Peds. (#/hr)	101		32	32		101			34			34
Confl. Bikes (#/hr)			6			9						
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	149	902	3	12	600	335	30	0	28	85	0	113
Shared Lane Traffic (%)												
Lane Group Flow (vph)	149	905	0	0	947	0	0	58	0	0	85	113
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		12			0			0			0	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15		9	15		9	15		9	15		9
Number of Detectors	1	2		1	2		1	2		1	2	1
Detector Template	Left	Thru		Left	Thru		Left	Thru		Left	Thru	Right
Leading Detector (ft)	20	100		20	100		20	100		20	100	20
Trailing Detector (ft)	0	0		0	0		0	0		0	0	0
Detector 1 Position(ft)	0	0		0	0		0	0		0	0	0
Detector 1 Size(ft)	20	6		20	6		20	6		20	6	20
Detector 1 Type	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	Cl+Ex
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	0.0
Detector 2 Position(ft)		94			94			94			94	
Detector 2 Size(ft)		6			6			6			6	
Detector 2 Type		Cl+Ex			Cl+Ex			Cl+Ex			Cl+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0			0.0	

Bally's Chicago Casino
305: Canal St/Jefferson St & Grand Ave

2026 Future with Entertainment District
Timing Plan: AM Peak Hour

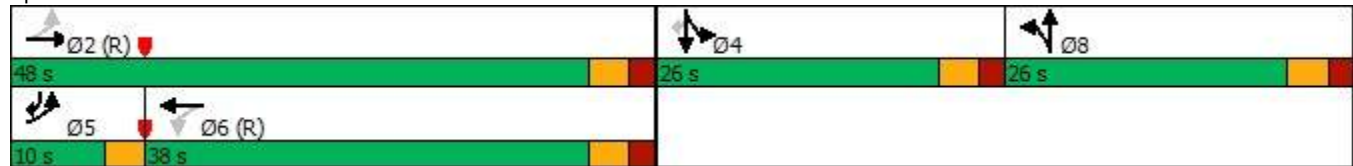


Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Turn Type	pm+pt	NA		Perm	NA		Split	NA		Split	NA	pm+ov
Protected Phases	5	2			6		8	8		4	4	5
Permitted Phases	2			6								4
Detector Phase	5	2		6	6		8	8		4	4	5
Switch Phase												
Minimum Initial (s)	5.0	5.0		5.0	5.0		5.0	5.0		5.0	5.0	5.0
Minimum Split (s)	10.0	26.0		26.0	26.0		26.0	26.0		23.0	23.0	10.0
Total Split (s)	10.0	48.0		38.0	38.0		26.0	26.0		26.0	26.0	10.0
Total Split (%)	10.0%	48.0%		38.0%	38.0%		26.0%	26.0%		26.0%	26.0%	10.0%
Maximum Green (s)	7.0	43.0		33.0	33.0		21.0	21.0		21.0	21.0	7.0
Yellow Time (s)	3.0	3.0		3.0	3.0		3.0	3.0		3.0	3.0	3.0
All-Red Time (s)	0.0	2.0		2.0	2.0		2.0	2.0		2.0	2.0	0.0
Lost Time Adjust (s)	0.0	0.0			0.0			0.0			0.0	0.0
Total Lost Time (s)	3.0	5.0			5.0			5.0			5.0	3.0
Lead/Lag	Lead			Lag	Lag							Lead
Lead-Lag Optimize?	Yes			Yes	Yes							Yes
Vehicle Extension (s)	3.0	3.0		3.0	3.0		3.0	3.0		3.0	3.0	3.0
Recall Mode	None	C-Max		C-Max	C-Max		None	None		None	None	None
Walk Time (s)		7.0		7.0	7.0		7.0	7.0				
Flash Dont Walk (s)		14.0		14.0	14.0		14.0	14.0				
Pedestrian Calls (#/hr)		0		0	0		0	0				
Act Effct Green (s)	75.6	74.6			61.8			5.6			10.1	19.3
Actuated g/C Ratio	0.76	0.75			0.62			0.06			0.10	0.19
v/c Ratio	0.33	0.34			0.52			0.30			0.47	0.29
Control Delay	6.2	4.7			12.9			5.1			50.5	5.9
Queue Delay	0.0	0.1			0.0			0.0			0.0	0.0
Total Delay	6.2	4.8			12.9			5.1			50.5	5.9
LOS	A	A			B			A			D	A
Approach Delay		5.0			12.9			5.1			25.0	
Approach LOS		A			B			A			C	
90th %ile Green (s)	12.0	64.9		49.9	49.9		6.0	6.0		14.1	14.1	12.0
90th %ile Term Code	Gap	Coord		Coord	Coord		Gap	Gap		Gap	Gap	Gap
70th %ile Green (s)	9.7	67.8		55.1	55.1		5.5	5.5		11.7	11.7	9.7
70th %ile Term Code	Gap	Coord		Coord	Coord		Gap	Gap		Gap	Gap	Gap
50th %ile Green (s)	8.7	69.4		57.7	57.7		5.5	5.5		10.1	10.1	8.7
50th %ile Term Code	Gap	Coord		Coord	Coord		Gap	Gap		Gap	Gap	Gap
30th %ile Green (s)	7.9	71.0		60.1	60.1		5.5	5.5		8.5	8.5	7.9
30th %ile Term Code	Gap	Coord		Coord	Coord		Gap	Gap		Gap	Gap	Gap
10th %ile Green (s)	6.0	95.0		86.0	86.0		0.0	0.0		0.0	0.0	6.0
10th %ile Term Code	Gap	Coord		Coord	Coord		Skip	Skip		Skip	Skip	Gap
Queue Length 50th (ft)	19	92			162			0			52	0
Queue Length 95th (ft)	37	101			266			7			96	32
Internal Link Dist (ft)		295			758			189			350	
Turn Bay Length (ft)	100											
Base Capacity (vph)	453	2640			1807			432			371	392
Starvation Cap Reductn	0	666			0			0			0	0
Spillback Cap Reductn	0	0			0			0			0	0
Storage Cap Reductn	0	0			0			0			0	0
Reduced v/c Ratio	0.33	0.46			0.52			0.13			0.23	0.29

Intersection Summary













Area Type:	Other
Cycle Length:	100
Actuated Cycle Length:	100
Offset:	0 (0%), Referenced to phase 2:EBTL and 6:WBTL, Start of Green
Natural Cycle:	85
Control Type:	Actuated-Coordinated
Maximum v/c Ratio:	0.52
Intersection Signal Delay:	10.1
Intersection LOS:	B
Intersection Capacity Utilization	78.6%
ICU Level of Service	D
Analysis Period (min)	15

Splits and Phases: 305: Canal St/Jefferson St & Grand Ave



Bally's Chicago Casino
400: Jefferson St & Garage Entrance

2026 Future with Entertainment District
Timing Plan: AM Peak Hour

						
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (vph)	103	202	176	202	118	122
Future Volume (vph)	103	202	176	202	118	122
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0	0		160	205	
Storage Lanes	1	1		0	0	
Taper Length (ft)	25				70	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor	0.82	0.95		0.95	0.98	
Frt		0.850		0.850		
Flt Protected	0.950				0.950	
Satd. Flow (prot)	1805	1615	1863	775	1805	1863
Flt Permitted	0.950				0.609	
Satd. Flow (perm)	1480	1534	1863	736	1133	1863
Right Turn on Red		Yes		Yes		
Satd. Flow (RTOR)		220		220		
Link Speed (mph)	30		30			30
Link Distance (ft)	244		120			363
Travel Time (s)	5.5		2.7			8.3
Confl. Peds. (#/hr)	59	10		10	10	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	0%	0%	2%	0%	0%	2%
Parking (#/hr)				84		
Adj. Flow (vph)	112	220	191	220	128	133
Shared Lane Traffic (%)						
Lane Group Flow (vph)	112	220	191	220	128	133
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Right	Left	Left
Median Width(ft)	12		12			12
Link Offset(ft)	0		0			0
Crosswalk Width(ft)	16		16			16
Two way Left Turn Lane						
Headway Factor	1.00	1.00	1.00	2.41	1.00	1.00
Turning Speed (mph)	15	9		9	15	
Number of Detectors	1	1	2	1	1	2
Detector Template	Left	Right	Thru	Right	Left	Thru
Leading Detector (ft)	20	20	100	20	20	100
Trailing Detector (ft)	0	0	0	0	0	0
Detector 1 Position(ft)	0	0	0	0	0	0
Detector 1 Size(ft)	20	20	6	20	20	6
Detector 1 Type	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex
Detector 1 Channel						
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 2 Position(ft)			94			94
Detector 2 Size(ft)			6			6
Detector 2 Type			Cl+Ex			Cl+Ex
Detector 2 Channel						



Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Detector 2 Extend (s)			0.0			0.0
Turn Type	Perm	pm+ov	NA	custom	pm+pt	NA
Protected Phases		1	2		1	6
Permitted Phases	8	8		2 8	6	
Detector Phase	8	1	2	2 8	1	6
Switch Phase						
Minimum Initial (s)	5.0	5.0	1.0		5.0	5.0
Minimum Split (s)	23.0	16.0	23.0		16.0	23.0
Total Split (s)	27.0	18.0	65.0		18.0	83.0
Total Split (%)	24.5%	16.4%	59.1%		16.4%	75.5%
Maximum Green (s)	22.0	15.0	60.0		15.0	78.0
Yellow Time (s)	3.0	3.0	3.0		3.0	3.0
All-Red Time (s)	2.0	0.0	2.0		0.0	2.0
Lost Time Adjust (s)	0.0	0.0	0.0		0.0	0.0
Total Lost Time (s)	5.0	3.0	5.0		3.0	5.0
Lead/Lag		Lead	Lag		Lead	
Lead-Lag Optimize?		Yes	Yes		Yes	
Vehicle Extension (s)	3.0	3.0	3.0		3.0	3.0
Recall Mode	Max	None	C-Max		None	C-Max
Walk Time (s)	7.0		7.0			7.0
Flash Dont Walk (s)	11.0		11.0			11.0
Pedestrian Calls (#/hr)	0		0			0
Act Effct Green (s)	22.0	31.8	67.2	94.2	80.0	78.0
Actuated g/C Ratio	0.20	0.29	0.61	0.86	0.73	0.71
v/c Ratio	0.38	0.36	0.17	0.33	0.15	0.10
Control Delay	42.5	5.4	9.9	1.6	5.3	5.8
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	42.5	5.4	9.9	1.6	5.3	5.8
LOS	D	A	A	A	A	A
Approach Delay	17.9		5.5			5.5
Approach LOS	B		A			A
90th %ile Green (s)	22.0	9.4	65.6		9.4	78.0
90th %ile Term Code	MaxR	Gap	Coord		Gap	Coord
70th %ile Green (s)	22.0	8.4	66.6		8.4	78.0
70th %ile Term Code	MaxR	Gap	Coord		Gap	Coord
50th %ile Green (s)	22.0	7.7	67.3		7.7	78.0
50th %ile Term Code	MaxR	Gap	Coord		Gap	Coord
30th %ile Green (s)	22.0	7.1	67.9		7.1	78.0
30th %ile Term Code	MaxR	Gap	Coord		Gap	Coord
10th %ile Green (s)	22.0	6.2	68.8		6.2	78.0
10th %ile Term Code	MaxR	Gap	Coord		Gap	Coord
Queue Length 50th (ft)	69	0	54	0	27	30
Queue Length 95th (ft)	125	52	91	10	45	47
Internal Link Dist (ft)	164		40			283
Turn Bay Length (ft)				160	205	
Base Capacity (vph)	296	696	1138	662	915	1321
Starvation Cap Reductn	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0

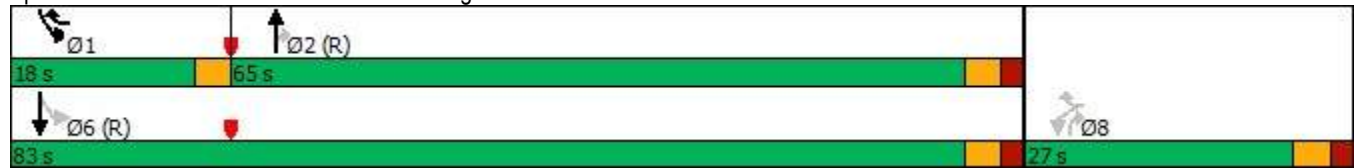


Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Reduced v/c Ratio	0.38	0.32	0.17	0.33	0.14	0.10

Intersection Summary

Area Type:	Other
Cycle Length:	110
Actuated Cycle Length:	110
Offset:	68 (62%), Referenced to phase 2:NBT and 6:SBTL, Start of Green
Natural Cycle:	65
Control Type:	Actuated-Coordinated
Maximum v/c Ratio:	0.38
Intersection Signal Delay:	9.6
Intersection LOS:	A
Intersection Capacity Utilization	48.2%
ICU Level of Service	A
Analysis Period (min)	15

Splits and Phases: 400: Jefferson St & Garage Entrance



Intersection						
Int Delay, s/veh	0.4					
Movement	WBL	WBR	SEL	SET	NWT	NWR
Lane Configurations		↗	↘	↑	↑	↗
Traffic Vol, veh/h	0	1	27	198	378	49
Future Vol, veh/h	0	1	27	198	378	49
Conflicting Peds, #/hr	0	0	10	0	0	30
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	-	0	100	-	-	50
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	0	2	2	0
Mvmt Flow	0	1	29	215	411	53

Major/Minor	Minor2	Major1	Major2			
Conflicting Flow All	-	441	494	0	-	0
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Critical Hdwy	-	6.22	4.1	-	-	-
Critical Hdwy Stg 1	-	-	-	-	-	-
Critical Hdwy Stg 2	-	-	-	-	-	-
Follow-up Hdwy	-	3.318	2.2	-	-	-
Pot Cap-1 Maneuver	0	616	1080	-	-	-
Stage 1	0	-	-	-	-	-
Stage 2	0	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	-	598	1049	-	-	-
Mov Cap-2 Maneuver	-	-	-	-	-	-
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-

Approach	WB	SE	NW
HCM Control Delay, s	11	1	0
HCM LOS	B		

Minor Lane/Major Mvmt	NWT	NWRWBLn1	SEL	SET
Capacity (veh/h)	-	-	598	1049
HCM Lane V/C Ratio	-	-	0.002	0.028
HCM Control Delay (s)	-	-	11	8.5
HCM Lane LOS	-	-	B	A
HCM 95th %tile Q(veh)	-	-	0	0.1

Intersection						
Int Delay, s/veh	1.4					
Movement	WBL	WBR	SEL	SET	NWT	NWR
Lane Configurations		↗	↖	↑	↘	
Traffic Vol, veh/h	0	57	29	166	374	52
Future Vol, veh/h	0	57	29	166	374	52
Conflicting Peds, #/hr	0	10	10	0	0	15
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	-	0	100	-	-	-
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	0	0	0	2	2	0
Mvmt Flow	0	62	32	180	407	57

Major/Minor	Minor2	Major1	Major2			
Conflicting Flow All	-	461	479	0	-	0
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Critical Hdwy	-	6.2	4.1	-	-	-
Critical Hdwy Stg 1	-	-	-	-	-	-
Critical Hdwy Stg 2	-	-	-	-	-	-
Follow-up Hdwy	-	3.3	2.2	-	-	-
Pot Cap-1 Maneuver	0	605	1094	-	-	-
Stage 1	0	-	-	-	-	-
Stage 2	0	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	-	588	1073	-	-	-
Mov Cap-2 Maneuver	-	-	-	-	-	-
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-

Approach	WB	SE	NW
HCM Control Delay, s	11.8	1.3	0
HCM LOS	B		













Minor Lane/Major Mvmt	NWT	NWRWBLn1	SEL	SET
Capacity (veh/h)	-	-	588	1073
HCM Lane V/C Ratio	-	-	0.105	0.029
HCM Control Delay (s)	-	-	11.8	8.5
HCM Lane LOS	-	-	B	A
HCM 95th %tile Q(veh)	-	-	0.4	0.1

Intersection						
Int Delay, s/veh	0					
Movement	WBL	WBR	SEL	SET	NWT	NWR
Lane Configurations						
Traffic Vol, veh/h	0	0	0	180	445	0
Future Vol, veh/h	0	0	0	180	445	0
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	100
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	0	0	0	196	484	0

Major/Minor	Minor2	Major1	Major2			
Conflicting Flow All	582	484	484	0	-	0
Stage 1	484	-	-	-	-	-
Stage 2	98	-	-	-	-	-
Critical Hdwy	6.63	6.23	4.13	-	-	-
Critical Hdwy Stg 1	5.43	-	-	-	-	-
Critical Hdwy Stg 2	5.83	-	-	-	-	-
Follow-up Hdwy	3.519	3.319	2.219	-	-	-
Pot Cap-1 Maneuver	459	582	1077	-	-	-
Stage 1	619	-	-	-	-	-
Stage 2	915	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	459	582	1077	-	-	-
Mov Cap-2 Maneuver	459	-	-	-	-	-
Stage 1	619	-	-	-	-	-
Stage 2	915	-	-	-	-	-

Approach	WB	SE	NW
HCM Control Delay, s	0	0	0
HCM LOS	A		

Minor Lane/Major Mvmt	NWT	NWRWBLn1	SEL	SET
Capacity (veh/h)	-	-	-	1077
HCM Lane V/C Ratio	-	-	-	-
HCM Control Delay (s)	-	-	0	0
HCM Lane LOS	-	-	A	A
HCM 95th %tile Q(veh)	-	-	-	0

						
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations						
Traffic Volume (vph)	1087	181	136	1329	182	411
Future Volume (vph)	1087	181	136	1329	182	411
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)		55	95		190	190
Storage Lanes		1	1		0	1
Taper Length (ft)			90		75	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor		0.90			0.86	0.94
Frt		0.850				0.850
Flt Protected			0.950		0.950	
Satd. Flow (prot)	1863	1583	1770	1863	1770	1583
Flt Permitted			0.056		0.950	
Satd. Flow (perm)	1863	1422	104	1863	1526	1489
Right Turn on Red		Yes				Yes
Satd. Flow (RTOR)		46				68
Link Speed (mph)	30			30	30	
Link Distance (ft)	390			893	363	
Travel Time (s)	8.9			20.3	8.3	
Confl. Peds. (#/hr)		50	50		50	50
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	1182	197	148	1445	198	447
Shared Lane Traffic (%)						
Lane Group Flow (vph)	1182	197	148	1445	198	447
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Left	Left	Right
Median Width(ft)	12			12	12	
Link Offset(ft)	0			0	0	
Crosswalk Width(ft)	16			16	16	
Two way Left Turn Lane						
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)		9	15		15	9
Number of Detectors	2	1	1	2	1	1
Detector Template	Thru	Right	Left	Thru	Left	Right
Leading Detector (ft)	100	20	20	100	20	20
Trailing Detector (ft)	0	0	0	0	0	0
Detector 1 Position(ft)	0	0	0	0	0	0
Detector 1 Size(ft)	6	20	20	6	20	20
Detector 1 Type	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex
Detector 1 Channel						
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 2 Position(ft)	94			94		
Detector 2 Size(ft)	6			6		
Detector 2 Type	Cl+Ex			Cl+Ex		
Detector 2 Channel						
Detector 2 Extend (s)	0.0			0.0		
Turn Type	NA	Perm	pm+pt	NA	Prot	pt+ov

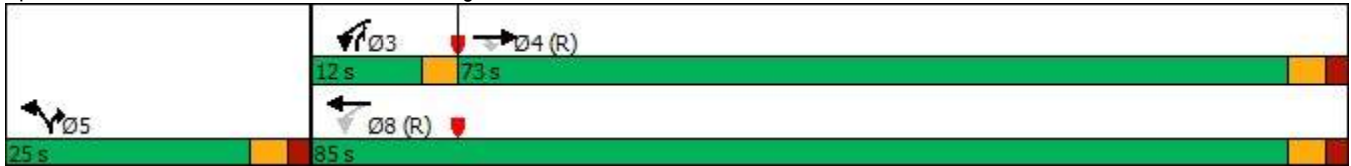


Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Protected Phases	4		3	8	5	5 3
Permitted Phases		4	8		5	5 3
Detector Phase	4	4	3	8	5	5 3
Switch Phase						
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	
Minimum Split (s)	23.0	23.0	9.5	23.0	10.0	
Total Split (s)	73.0	73.0	12.0	85.0	25.0	
Total Split (%)	66.4%	66.4%	10.9%	77.3%	22.7%	
Maximum Green (s)	68.0	68.0	9.0	80.0	20.0	
Yellow Time (s)	3.0	3.0	3.0	3.0	3.0	
All-Red Time (s)	2.0	2.0	0.0	2.0	2.0	
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	
Total Lost Time (s)	5.0	5.0	3.0	5.0	5.0	
Lead/Lag	Lag	Lag	Lead			
Lead-Lag Optimize?	Yes	Yes	Yes			
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	
Recall Mode	C-Max	C-Max	None	C-Max	Max	
Walk Time (s)	7.0	7.0		7.0		
Flash Dont Walk (s)	11.0	11.0		11.0		
Pedestrian Calls (#/hr)	0	0		0		
Act Effct Green (s)	68.0	68.0	82.0	80.0	20.0	32.0
Actuated g/C Ratio	0.62	0.62	0.75	0.73	0.18	0.29
v/c Ratio	1.03	0.22	0.69	1.07	0.62	0.88
Control Delay	55.8	7.7	25.6	62.8	50.8	56.6
Queue Delay	28.8	0.0	0.0	14.1	0.0	6.3
Total Delay	84.6	7.7	25.6	76.9	50.8	62.8
LOS	F	A	C	E	D	E
Approach Delay	73.6			72.1	59.1	
Approach LOS	E			E	E	
90th %ile Green (s)	68.0	68.0	9.0	80.0	20.0	
90th %ile Term Code	Coord	Coord	Max	Coord	MaxR	
70th %ile Green (s)	68.0	68.0	9.0	80.0	20.0	
70th %ile Term Code	Coord	Coord	Max	Coord	MaxR	
50th %ile Green (s)	68.0	68.0	9.0	80.0	20.0	
50th %ile Term Code	Coord	Coord	Max	Coord	MaxR	
30th %ile Green (s)	68.0	68.0	9.0	80.0	20.0	
30th %ile Term Code	Coord	Coord	Max	Coord	MaxR	
10th %ile Green (s)	68.0	68.0	9.0	80.0	20.0	
10th %ile Term Code	Coord	Coord	Max	Coord	MaxR	
Queue Length 50th (ft)	~893	42	29	~1161	140	284
Queue Length 95th (ft)	#1148	77	m40	m#1251	198	#455
Internal Link Dist (ft)	310			813	283	
Turn Bay Length (ft)		55	95		190	190
Base Capacity (vph)	1151	896	213	1354	321	508
Starvation Cap Reductn	242	0	0	210	0	36
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	1.30	0.22	0.69	1.26	0.62	0.95

Intersection Summary

Area Type:	Other
Cycle Length:	110
Actuated Cycle Length:	110
Offset:	13 (12%), Referenced to phase 4:EBT and 8:WBTL, Start of Green
Natural Cycle:	90
Control Type:	Actuated-Coordinated
Maximum v/c Ratio:	1.07
Intersection Signal Delay:	70.4
Intersection LOS:	E
Intersection Capacity Utilization:	95.0%
ICU Level of Service:	F
Analysis Period (min):	15
~	Volume exceeds capacity, queue is theoretically infinite. Queue shown is maximum after two cycles.
#	95th percentile volume exceeds capacity, queue may be longer. Queue shown is maximum after two cycles.
m	Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 204: Jefferson St & Chicago Ave



Bally's Chicago Casino
305: Canal St/Jefferson St & Grand Ave

2026 Future with Entertainment District
Timing Plan: PM Peak Hour



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	181	707	7	15	705	407	22	0	14	122	0	165
Future Volume (vph)	181	707	7	15	705	407	22	0	14	122	0	165
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	100		0	0		0	0		0	0		0
Storage Lanes	1		0	0		0	0		0	0		1
Taper Length (ft)	100			25			25			135		
Lane Util. Factor	1.00	0.95	0.95	0.95	0.95	0.95	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor		1.00			0.86			0.97			0.93	
Frt		0.998			0.946			0.948				0.850
Flt Protected	0.950				0.999			0.970			0.950	
Satd. Flow (prot)	1770	3528	0	0	2874	0	0	1660	0	0	1770	1583
Flt Permitted	0.130				0.940			0.970			0.950	
Satd. Flow (perm)	242	3528	0	0	2704	0	0	1660	0	0	1652	1583
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		1			118			109				179
Link Speed (mph)		30			30			30				30
Link Distance (ft)		375			838			269				430
Travel Time (s)		8.5			19.0			6.1				9.8
Confl. Peds. (#/hr)	149		35	35		149			49	49		
Confl. Bikes (#/hr)			2			30						
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	197	768	8	16	766	442	24	0	15	133	0	179
Shared Lane Traffic (%)												
Lane Group Flow (vph)	197	776	0	0	1224	0	0	39	0	0	133	179
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		12			0			0				0
Link Offset(ft)		0			0			0				0
Crosswalk Width(ft)		16			16			16				16
Two way Left Turn Lane												
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15		9	15		9	15		9	15		9
Number of Detectors	1	2		1	2		1	2		1	2	1
Detector Template	Left	Thru		Left	Thru		Left	Thru		Left	Thru	Right
Leading Detector (ft)	20	100		20	100		20	100		20	100	20
Trailing Detector (ft)	0	0		0	0		0	0		0	0	0
Detector 1 Position(ft)	0	0		0	0		0	0		0	0	0
Detector 1 Size(ft)	20	6		20	6		20	6		20	6	20
Detector 1 Type	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	Cl+Ex
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	0.0
Detector 2 Position(ft)		94			94			94				94
Detector 2 Size(ft)		6			6			6				6
Detector 2 Type		Cl+Ex			Cl+Ex			Cl+Ex				Cl+Ex
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0				0.0

Bally's Chicago Casino
305: Canal St/Jefferson St & Grand Ave

2026 Future with Entertainment District
Timing Plan: PM Peak Hour

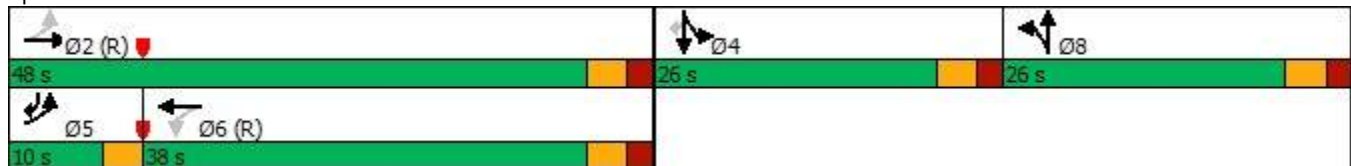


Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Turn Type	pm+pt	NA		Perm	NA		Split	NA		Split	NA	pm+ov
Protected Phases	5	2			6		8	8		4	4	5
Permitted Phases	2			6								4
Detector Phase	5	2		6	6		8	8		4	4	5
Switch Phase												
Minimum Initial (s)	5.0	5.0		5.0	5.0		5.0	5.0		5.0	5.0	5.0
Minimum Split (s)	10.0	26.0		26.0	26.0		26.0	26.0		23.0	23.0	10.0
Total Split (s)	10.0	48.0		38.0	38.0		26.0	26.0		26.0	26.0	10.0
Total Split (%)	10.0%	48.0%		38.0%	38.0%		26.0%	26.0%		26.0%	26.0%	10.0%
Maximum Green (s)	7.0	43.0		33.0	33.0		21.0	21.0		21.0	21.0	7.0
Yellow Time (s)	3.0	3.0		3.0	3.0		3.0	3.0		3.0	3.0	3.0
All-Red Time (s)	0.0	2.0		2.0	2.0		2.0	2.0		2.0	2.0	0.0
Lost Time Adjust (s)	0.0	0.0			0.0			0.0			0.0	0.0
Total Lost Time (s)	3.0	5.0			5.0			5.0			5.0	3.0
Lead/Lag	Lead			Lag	Lag							Lead
Lead-Lag Optimize?	Yes			Yes	Yes							Yes
Vehicle Extension (s)	3.0	3.0		3.0	3.0		3.0	3.0		3.0	3.0	3.0
Recall Mode	None	C-Max		C-Max	C-Max		None	None		None	None	None
Walk Time (s)		7.0		7.0	7.0		7.0	7.0				
Flash Dont Walk (s)		14.0		14.0	14.0		14.0	14.0				
Pedestrian Calls (#/hr)		0		0	0		0	0				
Act Effct Green (s)	72.9	70.9			54.9			5.5			12.8	29.0
Actuated g/C Ratio	0.73	0.71			0.55			0.06			0.13	0.29
v/c Ratio	0.53	0.31			0.80			0.20			0.59	0.31
Control Delay	22.6	5.7			23.7			2.3			51.1	3.8
Queue Delay	0.0	0.1			0.0			0.0			0.0	0.0
Total Delay	22.6	5.8			23.7			2.4			51.1	3.9
LOS	C	A			C			A			D	A
Approach Delay		9.2			23.7			2.4			24.0	
Approach LOS		A			C			A			C	
90th %ile Green (s)	16.2	61.8		42.6	42.6		5.5	5.5		17.7	17.7	16.2
90th %ile Term Code	Gap	Coord		Coord	Coord		Gap	Gap		Gap	Gap	Gap
70th %ile Green (s)	13.6	64.7		48.1	48.1		5.5	5.5		14.8	14.8	13.6
70th %ile Term Code	Gap	Coord		Coord	Coord		Gap	Gap		Gap	Gap	Gap
50th %ile Green (s)	12.4	66.7		51.3	51.3		5.5	5.5		12.8	12.8	12.4
50th %ile Term Code	Gap	Coord		Coord	Coord		Gap	Gap		Gap	Gap	Gap
30th %ile Green (s)	11.5	79.2		64.7	64.7		0.0	0.0		10.8	10.8	11.5
30th %ile Term Code	Gap	Coord		Coord	Coord		Skip	Skip		Gap	Gap	Gap
10th %ile Green (s)	11.5	82.1		67.6	67.6		0.0	0.0		7.9	7.9	11.5
10th %ile Term Code	Gap	Coord		Coord	Coord		Skip	Skip		Gap	Gap	Gap
Queue Length 50th (ft)	44	97			310			0			81	0
Queue Length 95th (ft)	142	102			#541			0			135	33
Internal Link Dist (ft)		295			758			189			350	
Turn Bay Length (ft)	100											
Base Capacity (vph)	375	2501			1536			434			371	586
Starvation Cap Reductn	0	702			0			0			0	0
Spillback Cap Reductn	0	0			3			33			0	20
Storage Cap Reductn	0	0			0			0			0	0
Reduced v/c Ratio	0.53	0.43			0.80			0.10			0.36	0.32

Intersection Summary

Area Type: Other
 Cycle Length: 100
 Actuated Cycle Length: 100
 Offset: 0 (0%), Referenced to phase 2:EBTL and 6:WBTL, Start of Green
 Natural Cycle: 85
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.80
 Intersection Signal Delay: 17.9 Intersection LOS: B
 Intersection Capacity Utilization 83.9% ICU Level of Service E
 Analysis Period (min) 15
 # 95th percentile volume exceeds capacity, queue may be longer.
 Queue shown is maximum after two cycles.

Splits and Phases: 305: Canal St/Jefferson St & Grand Ave





Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (vph)	170	329	264	257	149	168
Future Volume (vph)	170	329	264	257	149	168
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0	0		160	205	
Storage Lanes	1	1		0	0	
Taper Length (ft)	25				70	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor	0.74	0.92		0.92	0.97	
Flt		0.850		0.850		
Flt Protected	0.950				0.950	
Satd. Flow (prot)	1805	1615	1863	646	1805	1863
Flt Permitted	0.950				0.526	
Satd. Flow (perm)	1342	1486	1863	594	967	1863
Right Turn on Red		Yes		Yes		
Satd. Flow (RTOR)		358		279		
Link Speed (mph)	30		30			30
Link Distance (ft)	244		120			363
Travel Time (s)	5.5		2.7			8.3
Confl. Peds. (#/hr)	84	20		20	20	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	0%	0%	2%	0%	0%	2%
Parking (#/hr)				100		
Adj. Flow (vph)	185	358	287	279	162	183
Shared Lane Traffic (%)						
Lane Group Flow (vph)	185	358	287	279	162	183
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Right	Left	Left
Median Width(ft)	12		12			12
Link Offset(ft)	0		0			0
Crosswalk Width(ft)	16		16			16
Two way Left Turn Lane						
Headway Factor	1.00	1.00	1.00	2.95	1.00	1.00
Turning Speed (mph)	15	9		9	15	
Number of Detectors	1	1	2	1	1	2
Detector Template	Left	Right	Thru	Right	Left	Thru
Leading Detector (ft)	20	20	100	20	20	100
Trailing Detector (ft)	0	0	0	0	0	0
Detector 1 Position(ft)	0	0	0	0	0	0
Detector 1 Size(ft)	20	20	6	20	20	6
Detector 1 Type	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex
Detector 1 Channel						
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 2 Position(ft)			94			94
Detector 2 Size(ft)			6			6
Detector 2 Type			Cl+Ex			Cl+Ex
Detector 2 Channel						



Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Detector 2 Extend (s)			0.0			0.0
Turn Type	Perm	pm+ov	NA	custom	pm+pt	NA
Protected Phases		1	2		1	6
Permitted Phases	8	8		2 8	6	
Detector Phase	8	1	2	2 8	1	6
Switch Phase						
Minimum Initial (s)	5.0	5.0	5.0		5.0	5.0
Minimum Split (s)	23.0	16.0	23.0		16.0	23.0
Total Split (s)	29.0	17.0	64.0		17.0	81.0
Total Split (%)	26.4%	15.5%	58.2%		15.5%	73.6%
Maximum Green (s)	24.0	14.0	59.0		14.0	76.0
Yellow Time (s)	3.0	3.0	3.0		3.0	3.0
All-Red Time (s)	2.0	0.0	2.0		0.0	2.0
Lost Time Adjust (s)	0.0	0.0	0.0		0.0	0.0
Total Lost Time (s)	5.0	3.0	5.0		3.0	5.0
Lead/Lag		Lead	Lag		Lead	
Lead-Lag Optimize?		Yes	Yes		Yes	
Vehicle Extension (s)	3.0	3.0	3.0		3.0	3.0
Recall Mode	Max	None	C-Max		None	C-Max
Walk Time (s)	7.0		7.0			7.0
Flash Dont Walk (s)	11.0		11.0			11.0
Pedestrian Calls (#/hr)	0		0			0
Act Effct Green (s)	24.0	34.8	64.2	93.2	78.0	76.0
Actuated g/C Ratio	0.22	0.32	0.58	0.85	0.71	0.69
v/c Ratio	0.63	0.49	0.26	0.51	0.22	0.14
Control Delay	50.0	5.1	12.4	3.7	2.4	2.5
Queue Delay	0.0	0.8	0.0	0.0	0.0	0.0
Total Delay	50.0	5.9	12.4	3.7	2.4	2.5
LOS	D	A	B	A	A	A
Approach Delay	20.9		8.1			2.5
Approach LOS	C		A			A
90th %ile Green (s)	24.0	11.7	61.3		11.7	76.0
90th %ile Term Code	MaxR	Gap	Coord		Gap	Coord
70th %ile Green (s)	24.0	9.4	63.6		9.4	76.0
70th %ile Term Code	MaxR	Gap	Coord		Gap	Coord
50th %ile Green (s)	24.0	8.6	64.4		8.6	76.0
50th %ile Term Code	MaxR	Gap	Coord		Gap	Coord
30th %ile Green (s)	24.0	7.8	65.2		7.8	76.0
30th %ile Term Code	MaxR	Gap	Coord		Gap	Coord
10th %ile Green (s)	24.0	6.7	66.3		6.7	76.0
10th %ile Term Code	MaxR	Gap	Coord		Gap	Coord
Queue Length 50th (ft)	119	0	93	0	10	14
Queue Length 95th (ft)	198	59	152	13	m12	m17
Internal Link Dist (ft)	164		40			283
Turn Bay Length (ft)				160	205	
Base Capacity (vph)	292	784	1086	545	792	1287
Starvation Cap Reductn	0	0	0	0	0	0
Spillback Cap Reductn	0	186	64	0	0	0
Storage Cap Reductn	0	0	0	0	0	0



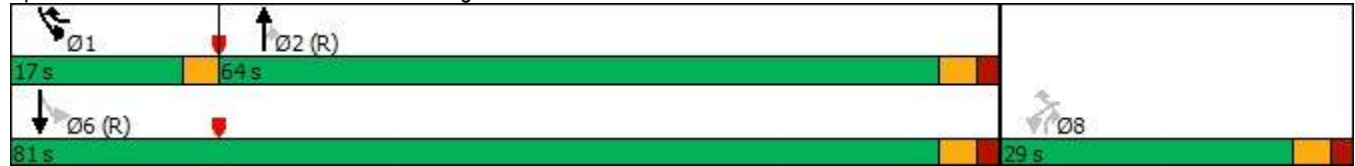
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Reduced v/c Ratio	0.63	0.60	0.28	0.51	0.20	0.14

Intersection Summary

Area Type:	Other
Cycle Length:	110
Actuated Cycle Length:	110
Offset:	22 (20%), Referenced to phase 2:NBT and 6:SBTL, Start of Green
Natural Cycle:	65
Control Type:	Actuated-Coordinated
Maximum v/c Ratio:	0.63
Intersection Signal Delay:	11.6
Intersection LOS:	B
Intersection Capacity Utilization	49.9%
ICU Level of Service	A
Analysis Period (min)	15

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 400: Jefferson St & Garage Entrance



Intersection						
Int Delay, s/veh	0.3					
Movement	WBL	WBR	SEL	SET	NWT	NWR
Lane Configurations		↗	↘	↖	↗	↘
Traffic Vol, veh/h	0	1	34	304	521	64
Future Vol, veh/h	0	1	34	304	521	64
Conflicting Peds, #/hr	0	0	20	0	0	42
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	-	0	100	-	-	50
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	0	2	2	0
Mvmt Flow	0	1	37	330	566	70

Major/Minor	Minor2	Major1	Major2		
Conflicting Flow All	-	608	678	0	-
Stage 1	-	-	-	-	-
Stage 2	-	-	-	-	-
Critical Hdwy	-	6.22	4.1	-	-
Critical Hdwy Stg 1	-	-	-	-	-
Critical Hdwy Stg 2	-	-	-	-	-
Follow-up Hdwy	-	3.318	2.2	-	-
Pot Cap-1 Maneuver	0	496	923	-	-
Stage 1	0	-	-	-	-
Stage 2	0	-	-	-	-
Platoon blocked, %				-	-
Mov Cap-1 Maneuver	-	476	886	-	-
Mov Cap-2 Maneuver	-	-	-	-	-
Stage 1	-	-	-	-	-
Stage 2	-	-	-	-	-

Approach	WB	SE	NW
HCM Control Delay, s	12.6	0.9	0
HCM LOS	B		

Minor Lane/Major Mvmt	NWT	NWRWBLn1	SEL	SET
Capacity (veh/h)	-	-	476	886
HCM Lane V/C Ratio	-	-	0.002	0.042
HCM Control Delay (s)	-	-	12.6	9.2
HCM Lane LOS	-	-	B	A
HCM 95th %tile Q(veh)	-	-	0	0.1

Intersection						
Int Delay, s/veh	1.8					
Movement	WBL	WBR	SEL	SET	NWT	NWR
Lane Configurations						
Traffic Vol, veh/h	0	91	39	265	495	69
Future Vol, veh/h	0	91	39	265	495	69
Conflicting Peds, #/hr	0	20	20	0	0	21
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	-	0	100	-	-	-
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	0	0	0	2	2	0
Mvmt Flow	0	99	42	288	538	75

Major/Minor	Minor2	Major1	Major2			
Conflicting Flow All	-	617	634	0	-	0
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Critical Hdwy	-	6.2	4.1	-	-	-
Critical Hdwy Stg 1	-	-	-	-	-	-
Critical Hdwy Stg 2	-	-	-	-	-	-
Follow-up Hdwy	-	3.3	2.2	-	-	-
Pot Cap-1 Maneuver	0	494	959	-	-	-
Stage 1	0	-	-	-	-	-
Stage 2	0	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	-	472	933	-	-	-
Mov Cap-2 Maneuver	-	-	-	-	-	-
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-

Approach	WB	SE	NW
HCM Control Delay, s	14.6	1.2	0
HCM LOS	B		

Minor Lane/Major Mvmt	NWT	NWRWBLn1	SEL	SET
Capacity (veh/h)	-	-	472	933
HCM Lane V/C Ratio	-	-	0.21	0.045
HCM Control Delay (s)	-	-	14.6	9
HCM Lane LOS	-	-	B	A
HCM 95th %tile Q(veh)	-	-	0.8	0.1

Intersection						
Int Delay, s/veh	0					
Movement	WBL	WBR	SEL	SET	NWT	NWR
Lane Configurations						
Traffic Vol, veh/h	0	0	0	288	588	0
Future Vol, veh/h	0	0	0	288	588	0
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	100
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	0	0	0	313	639	0

Major/Minor	Minor2	Major1	Major2			
Conflicting Flow All	796	639	639	0	-	0
Stage 1	639	-	-	-	-	-
Stage 2	157	-	-	-	-	-
Critical Hdwy	6.63	6.23	4.13	-	-	-
Critical Hdwy Stg 1	5.43	-	-	-	-	-
Critical Hdwy Stg 2	5.83	-	-	-	-	-
Follow-up Hdwy	3.519	3.319	2.219	-	-	-
Pot Cap-1 Maneuver	340	475	943	-	-	-
Stage 1	525	-	-	-	-	-
Stage 2	856	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	340	475	943	-	-	-
Mov Cap-2 Maneuver	340	-	-	-	-	-
Stage 1	525	-	-	-	-	-
Stage 2	856	-	-	-	-	-

Approach	WB	SE	NW
HCM Control Delay, s	0	0	0
HCM LOS	A		

Minor Lane/Major Mvmt	NWT	NWRWBLn1	SEL	SET
Capacity (veh/h)	-	-	-	943
HCM Lane V/C Ratio	-	-	-	-
HCM Control Delay (s)	-	-	0	0
HCM Lane LOS	-	-	A	A
HCM 95th %tile Q(veh)	-	-	-	0



Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↑	↑	↑	↑	↑	↑
Traffic Volume (vph)	811	238	179	576	233	525
Future Volume (vph)	811	238	179	576	233	525
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)		55	95		190	190
Storage Lanes		1	1		0	1
Taper Length (ft)			90		75	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor		0.90			0.91	0.96
Frt		0.850				0.850
Flt Protected			0.950		0.950	
Satd. Flow (prot)	1863	1583	1770	1863	1770	1583
Flt Permitted			0.072		0.950	
Satd. Flow (perm)	1863	1422	134	1863	1613	1519
Right Turn on Red		Yes				Yes
Satd. Flow (RTOR)		53				47
Link Speed (mph)	30			30	30	
Link Distance (ft)	390			893	363	
Travel Time (s)	8.9			20.3	8.3	
Confl. Peds. (#/hr)		50	50		50	50
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	882	259	195	626	253	571
Shared Lane Traffic (%)						
Lane Group Flow (vph)	882	259	195	626	253	571
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Left	Left	Right
Median Width(ft)	12			12	12	
Link Offset(ft)	0			0	0	
Crosswalk Width(ft)	16			16	16	
Two way Left Turn Lane						
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Number of Detectors	2	1	1	2	1	1
Detector Template	Thru	Right	Left	Thru	Left	Right
Leading Detector (ft)	100	20	20	100	20	20
Trailing Detector (ft)	0	0	0	0	0	0
Detector 1 Position(ft)	0	0	0	0	0	0
Detector 1 Size(ft)	6	20	20	6	20	20
Detector 1 Type	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex
Detector 1 Channel						
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 2 Position(ft)	94			94		
Detector 2 Size(ft)	6			6		
Detector 2 Type	Cl+Ex			Cl+Ex		
Detector 2 Channel						
Detector 2 Extend (s)	0.0			0.0		
Turn Type	NA	Perm	pm+pt	NA	Prot	pt+ov
Protected Phases	4		3	8	5	5 3

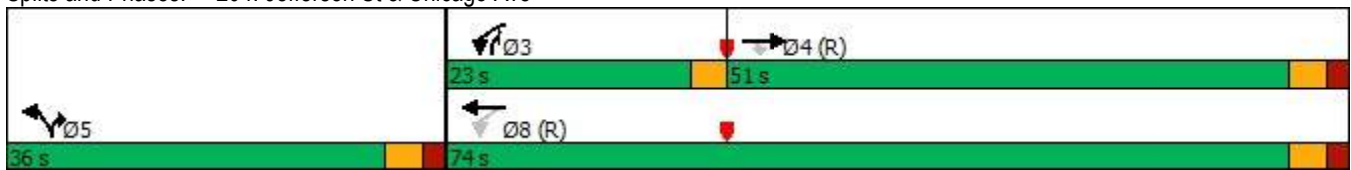


Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Permitted Phases		4	8		5	5 3
Detector Phase	4	4	3	8	5	5 3
Switch Phase						
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	
Minimum Split (s)	44.0	44.0	16.0	54.0	16.0	
Total Split (s)	51.0	51.0	23.0	74.0	36.0	
Total Split (%)	46.4%	46.4%	20.9%	67.3%	32.7%	
Maximum Green (s)	46.0	46.0	20.0	69.0	31.0	
Yellow Time (s)	3.0	3.0	3.0	3.0	3.0	
All-Red Time (s)	2.0	2.0	0.0	2.0	2.0	
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	
Total Lost Time (s)	5.0	5.0	3.0	5.0	5.0	
Lead/Lag	Lag	Lag	Lead			
Lead-Lag Optimize?	Yes	Yes	Yes			
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	
Recall Mode	C-Max	C-Max	None	C-Max	Max	
Walk Time (s)	7.0	7.0		7.0		
Flash Dont Walk (s)	11.0	11.0		11.0		
Pedestrian Calls (#/hr)	0	0		0		
Act Effct Green (s)	52.6	52.6	71.0	69.0	31.0	47.4
Actuated g/C Ratio	0.48	0.48	0.65	0.63	0.28	0.43
v/c Ratio	0.99	0.37	0.68	0.54	0.51	0.81
Control Delay	48.5	15.0	33.3	13.6	33.4	27.5
Queue Delay	37.2	0.6	0.0	1.4	1.0	0.8
Total Delay	85.8	15.6	33.3	15.0	34.4	28.2
LOS	F	B	C	B	C	C
Approach Delay	69.8			19.3	30.1	
Approach LOS	E			B	C	
90th %ile Green (s)	46.0	46.0	20.0	69.0	31.0	
90th %ile Term Code	Coord	Coord	Max	Coord	MaxR	
70th %ile Green (s)	48.6	48.6	17.4	69.0	31.0	
70th %ile Term Code	Coord	Coord	Gap	Coord	MaxR	
50th %ile Green (s)	53.1	53.1	12.9	69.0	31.0	
50th %ile Term Code	Coord	Coord	Gap	Coord	MaxR	
30th %ile Green (s)	56.9	56.9	9.1	69.0	31.0	
30th %ile Term Code	Coord	Coord	Gap	Coord	MaxR	
10th %ile Green (s)	58.4	58.4	7.6	69.0	31.0	
10th %ile Term Code	Coord	Coord	Gap	Coord	MaxR	
Queue Length 50th (ft)	534	76	78	229	113	201
Queue Length 95th (ft)	m#933	m120	148	323	200	338
Internal Link Dist (ft)	310			813	283	
Turn Bay Length (ft)		55	95		190	190
Base Capacity (vph)	891	707	383	1168	498	801
Starvation Cap Reductn	99	181	0	0	91	62
Spillback Cap Reductn	0	0	0	331	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	1.11	0.49	0.51	0.75	0.62	0.77

Intersection Summary

Area Type: Other
 Cycle Length: 110
 Actuated Cycle Length: 110
 Offset: 18 (16%), Referenced to phase 4:EBT and 8:WBTL, Start of Green
 Natural Cycle: 80
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.99
 Intersection Signal Delay: 43.2 Intersection LOS: D
 Intersection Capacity Utilization 87.5% ICU Level of Service E
 Analysis Period (min) 15
 # 95th percentile volume exceeds capacity, queue may be longer.
 Queue shown is maximum after two cycles.
 m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 204: Jefferson St & Chicago Ave



Bally's Chicago Casino
305: Canal St/Jefferson St & Grand Ave

2026 Future with Entertainment District
Timing Plan: Friday Casino PH



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	238	509	5	10	444	536	19	0	10	157	0	209
Future Volume (vph)	238	509	5	10	444	536	19	0	10	157	0	209
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	100		0	0		0	0		0	0		0
Storage Lanes	1		0	0		0	0		0	0		1
Taper Length (ft)	100			25			25			135		
Lane Util. Factor	1.00	0.95	0.95	0.95	0.95	0.95	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor		1.00			0.92			0.99			0.98	
Frt		0.999			0.919			0.954				0.850
Flt Protected	0.950				0.999			0.968			0.950	
Satd. Flow (prot)	1770	3534	0	0	2981	0	0	1698	0	0	1770	1583
Flt Permitted	0.138				0.948			0.968			0.950	
Satd. Flow (perm)	257	3534	0	0	2828	0	0	1698	0	0	1726	1583
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		1			310			109				227
Link Speed (mph)		30			30			30				30
Link Distance (ft)		375			838			269				430
Travel Time (s)		8.5			19.0			6.1				9.8
Confl. Peds. (#/hr)	56		17	17		56			18	18		
Confl. Bikes (#/hr)						2						
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	259	553	5	11	483	583	21	0	11	171	0	227
Shared Lane Traffic (%)												
Lane Group Flow (vph)	259	558	0	0	1077	0	0	32	0	0	171	227
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		12			0			0				0
Link Offset(ft)		0			0			0				0
Crosswalk Width(ft)		16			16			16				16
Two way Left Turn Lane												
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Number of Detectors	1	2		1	2		1	2		1	2	1
Detector Template	Left	Thru		Left	Thru		Left	Thru		Left	Thru	Right
Leading Detector (ft)	20	100		20	100		20	100		20	100	20
Trailing Detector (ft)	0	0		0	0		0	0		0	0	0
Detector 1 Position(ft)	0	0		0	0		0	0		0	0	0
Detector 1 Size(ft)	20	6		20	6		20	6		20	6	20
Detector 1 Type	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	Cl+Ex
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	0.0
Detector 2 Position(ft)		94			94			94				94
Detector 2 Size(ft)		6			6			6				6
Detector 2 Type		Cl+Ex			Cl+Ex			Cl+Ex				Cl+Ex
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0				0.0
Turn Type	pm+pt	NA		Perm	NA		Split	NA		Split	NA	pm+ov

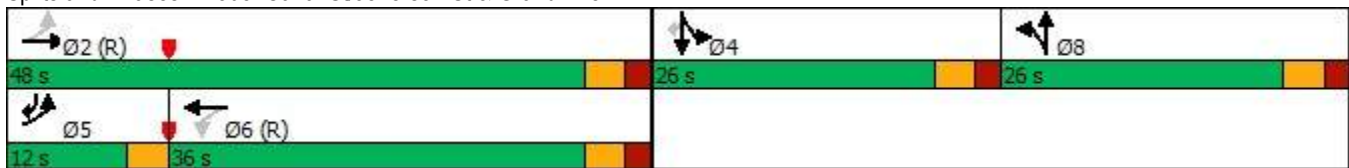


Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Protected Phases	5	2			6		8	8		4	4	5
Permitted Phases	2			6								4
Detector Phase	5	2		6	6		8	8		4	4	5
Switch Phase												
Minimum Initial (s)	5.0	5.0		5.0	5.0		5.0	5.0		5.0	5.0	5.0
Minimum Split (s)	10.0	26.0		26.0	26.0		26.0	26.0		23.0	23.0	10.0
Total Split (s)	12.0	48.0		36.0	36.0		26.0	26.0		26.0	26.0	12.0
Total Split (%)	12.0%	48.0%		36.0%	36.0%		26.0%	26.0%		26.0%	26.0%	12.0%
Maximum Green (s)	9.0	43.0		31.0	31.0		21.0	21.0		21.0	21.0	9.0
Yellow Time (s)	3.0	3.0		3.0	3.0		3.0	3.0		3.0	3.0	3.0
All-Red Time (s)	0.0	2.0		2.0	2.0		2.0	2.0		2.0	2.0	0.0
Lost Time Adjust (s)	0.0	0.0			0.0			0.0			0.0	0.0
Total Lost Time (s)	3.0	5.0			5.0			5.0			5.0	3.0
Lead/Lag	Lead			Lag	Lag							Lead
Lead-Lag Optimize?	Yes			Yes	Yes							Yes
Vehicle Extension (s)	3.0	3.0		3.0	3.0		3.0	3.0		3.0	3.0	3.0
Recall Mode	None	C-Max		C-Max	C-Max		None	None		None	None	None
Walk Time (s)		7.0		7.0	7.0		7.0	7.0				
Flash Dont Walk (s)		14.0		14.0	14.0		14.0	14.0				
Pedestrian Calls (#/hr)		0		0	0		0	0				
Act Effct Green (s)	70.8	68.8			47.3			5.5			14.9	36.6
Actuated g/C Ratio	0.71	0.69			0.47			0.06			0.15	0.37
v/c Ratio	0.56	0.23			0.72			0.16			0.65	0.31
Control Delay	11.4	6.3			19.9			1.8			51.2	2.8
Queue Delay	0.0	0.2			0.0			0.0			0.0	0.0
Total Delay	11.4	6.4			19.9			1.8			51.2	2.8
LOS	B	A			B			A			D	A
Approach Delay		8.0			19.9			1.8			23.6	
Approach LOS		A			B			A			C	
90th %ile Green (s)	21.7	59.1		34.4	34.4		5.5	5.5		20.4	20.4	21.7
90th %ile Term Code	Gap	Coord		Coord	Coord		Gap	Gap		Gap	Gap	Gap
70th %ile Green (s)	18.3	62.3		41.0	41.0		5.5	5.5		17.2	17.2	18.3
70th %ile Term Code	Gap	Coord		Coord	Coord		Gap	Gap		Gap	Gap	Gap
50th %ile Green (s)	17.2	64.6		44.4	44.4		5.5	5.5		14.9	14.9	17.2
50th %ile Term Code	Gap	Coord		Coord	Coord		Gap	Gap		Gap	Gap	Gap
30th %ile Green (s)	15.8	77.4		58.6	58.6		0.0	0.0		12.6	12.6	15.8
30th %ile Term Code	Gap	Coord		Coord	Coord		Skip	Skip		Gap	Gap	Gap
10th %ile Green (s)	19.6	80.7		58.1	58.1		0.0	0.0		9.3	9.3	19.6
10th %ile Term Code	Gap	Coord		Coord	Coord		Skip	Skip		Gap	Gap	Gap
Queue Length 50th (ft)	57	68			213			0			104	0
Queue Length 95th (ft)	96	93			#399			0			163	30
Internal Link Dist (ft)		295			758			189			350	
Turn Bay Length (ft)	100											
Base Capacity (vph)	462	2432			1500			442			371	723
Starvation Cap Reductn	0	958			0			0			0	0
Spillback Cap Reductn	0	0			0			0			0	0
Storage Cap Reductn	0	0			0			0			0	0
Reduced v/c Ratio	0.56	0.38			0.72			0.07			0.46	0.31

Intersection Summary

Area Type:	Other
Cycle Length:	100
Actuated Cycle Length:	100
Offset:	0 (0%), Referenced to phase 2:EBTL and 6:WBTL, Start of Green
Natural Cycle:	85
Control Type:	Actuated-Coordinated
Maximum v/c Ratio:	0.72
Intersection Signal Delay:	16.1
Intersection LOS:	B
Intersection Capacity Utilization	74.9%
ICU Level of Service	D
Analysis Period (min)	15
# 95th percentile volume exceeds capacity, queue may be longer.	
Queue shown is maximum after two cycles.	

Splits and Phases: 305: Canal St/Jefferson St & Grand Ave





Lane Group	WBL	WBR	NBT	NBR	SBL	SBT	Ø8
Lane Configurations							
Traffic Volume (vph)	216	417	341	340	199	218	
Future Volume (vph)	216	417	341	340	199	218	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	
Storage Length (ft)	0	0		160	205		
Storage Lanes	1	1		0	0		
Taper Length (ft)	25				70		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	
Ped Bike Factor	0.67	0.95		0.89	0.97		
Frt		0.850		0.850			
Flt Protected	0.950				0.950		
Satd. Flow (prot)	1805	1615	1863	646	1805	1500	
Flt Permitted	0.950				0.412		
Satd. Flow (perm)	1204	1541	1863	575	756	1500	
Right Turn on Red		Yes		Yes			
Satd. Flow (RTOR)		319		370			
Link Speed (mph)	30		30			30	
Link Distance (ft)	244		120			363	
Travel Time (s)	5.5		2.7			8.3	
Confl. Peds. (#/hr)	109	20		30	30		
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	
Heavy Vehicles (%)	0%	0%	2%	0%	0%	2%	
Parking (#/hr)				100		19	
Adj. Flow (vph)	235	453	371	370	216	237	
Shared Lane Traffic (%)							
Lane Group Flow (vph)	235	453	371	370	216	237	
Enter Blocked Intersection	No	No	No	No	No	No	
Lane Alignment	Left	Right	Left	Right	Left	Left	
Median Width(ft)	12		12			12	
Link Offset(ft)	0		0			0	
Crosswalk Width(ft)	16		16			16	
Two way Left Turn Lane							
Headway Factor	1.00	1.00	1.00	2.95	1.00	1.31	
Number of Detectors	1	1	2	1	1	2	
Detector Template	Left	Right	Thru	Right	Left	Thru	
Leading Detector (ft)	20	20	100	20	20	100	
Trailing Detector (ft)	0	0	0	0	0	0	
Detector 1 Position(ft)	0	0	0	0	0	0	
Detector 1 Size(ft)	20	20	6	20	20	6	
Detector 1 Type	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	
Detector 1 Channel							
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0	
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0	
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0	
Detector 2 Position(ft)			94			94	
Detector 2 Size(ft)			6			6	
Detector 2 Type			Cl+Ex			Cl+Ex	
Detector 2 Channel							
Detector 2 Extend (s)			0.0			0.0	

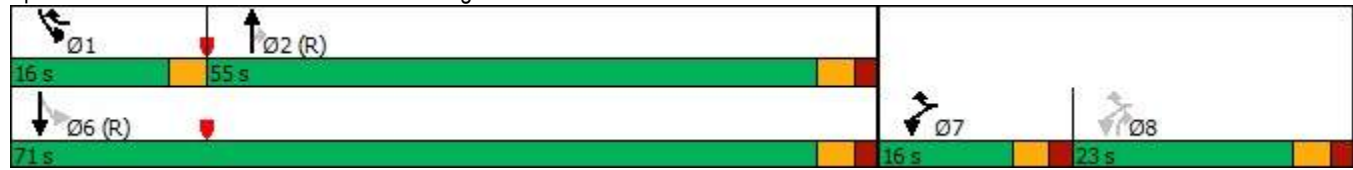


Lane Group	WBL	WBR	NBT	NBR	SBL	SBT	Ø8
Turn Type	custom	pt+ov	NA	custom	pm+pt	NA	
Protected Phases	7	17	2		1	6	8
Permitted Phases	8	7 8		2 8	6		
Detector Phase	7	17	2	2 8	1	6	
Switch Phase							
Minimum Initial (s)	5.0		5.0		5.0	5.0	5.0
Minimum Split (s)	16.0		23.0		16.0	23.0	23.0
Total Split (s)	16.0		55.0		16.0	71.0	23.0
Total Split (%)	14.5%		50.0%		14.5%	64.5%	21%
Maximum Green (s)	11.0		50.0		13.0	66.0	18.0
Yellow Time (s)	3.0		3.0		3.0	3.0	3.0
All-Red Time (s)	2.0		2.0		0.0	2.0	2.0
Lost Time Adjust (s)	0.0		0.0		0.0	0.0	
Total Lost Time (s)	5.0		5.0		3.0	5.0	
Lead/Lag	Lead		Lag		Lead		Lag
Lead-Lag Optimize?	Yes		Yes		Yes		Yes
Vehicle Extension (s)	3.0		3.0		3.0	3.0	3.0
Recall Mode	None		C-Max		None	C-Max	Max
Walk Time (s)			7.0			7.0	7.0
Flash Dont Walk (s)			11.0			11.0	11.0
Pedestrian Calls (#/hr)			0			0	0
Act Effct Green (s)	29.0	44.0	52.0	70.0	68.0	66.0	
Actuated g/C Ratio	0.26	0.40	0.47	0.64	0.62	0.60	
v/c Ratio	0.62	0.55	0.42	0.74	0.38	0.26	
Control Delay	39.5	8.4	21.4	11.2	16.2	16.8	
Queue Delay	0.0	0.0	0.2	0.0	0.0	0.9	
Total Delay	39.5	8.5	21.5	11.2	16.2	17.7	
LOS	D	A	C	B	B	B	
Approach Delay	19.1		16.4			17.0	
Approach LOS	B		B			B	
90th %ile Green (s)	11.0		50.0		13.0	66.0	18.0
90th %ile Term Code	Max		Coord		Max	Coord	MaxR
70th %ile Green (s)	11.0		50.0		13.0	66.0	18.0
70th %ile Term Code	Max		Coord		Max	Coord	MaxR
50th %ile Green (s)	11.0		51.8		11.2	66.0	18.0
50th %ile Term Code	Max		Coord		Gap	Coord	MaxR
30th %ile Green (s)	11.0		53.1		9.9	66.0	18.0
30th %ile Term Code	Max		Coord		Gap	Coord	MaxR
10th %ile Green (s)	11.0		54.9		8.1	66.0	18.0
10th %ile Term Code	Max		Coord		Gap	Coord	MaxR
Queue Length 50th (ft)	131	55	168	0	96	113	
Queue Length 95th (ft)	205	132	254	45	142	166	
Internal Link Dist (ft)	164		40			283	
Turn Bay Length (ft)				160	205		
Base Capacity (vph)	377	847	879	500	591	900	
Starvation Cap Reductn	0	0	0	0	0	423	
Spillback Cap Reductn	0	15	87	0	0	0	
Storage Cap Reductn	0	0	0	0	0	0	
Reduced v/c Ratio	0.62	0.54	0.47	0.74	0.37	0.50	

Intersection Summary

Area Type:	Other
Cycle Length:	110
Actuated Cycle Length:	110
Offset:	77 (70%), Referenced to phase 2:NBT and 6:SBTL, Start of Green
Natural Cycle:	80
Control Type:	Actuated-Coordinated
Maximum v/c Ratio:	0.74
Intersection Signal Delay:	17.5
Intersection LOS:	B
Intersection Capacity Utilization	53.2%
ICU Level of Service	A
Analysis Period (min)	15

Splits and Phases: 400: Jefferson St & Garage Entrance



Intersection						
Int Delay, s/veh	0.4					
Movement	WBL	WBR	SEL	SET	NWT	NWR
Lane Configurations		↗	↖	↑	↑	↗
Traffic Vol, veh/h	0	1	45	389	681	84
Future Vol, veh/h	0	1	45	389	681	84
Conflicting Peds, #/hr	0	0	30	0	0	54
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	-	0	100	-	-	50
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	0	2	2	0
Mvmt Flow	0	1	49	423	740	91

Major/Minor	Minor2	Major1	Major2			
Conflicting Flow All	-	794	885	0	-	0
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Critical Hdwy	-	6.22	4.1	-	-	-
Critical Hdwy Stg 1	-	-	-	-	-	-
Critical Hdwy Stg 2	-	-	-	-	-	-
Follow-up Hdwy	-	3.318	2.2	-	-	-
Pot Cap-1 Maneuver	0	388	773	-	-	-
Stage 1	0	-	-	-	-	-
Stage 2	0	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	-	368	733	-	-	-
Mov Cap-2 Maneuver	-	-	-	-	-	-
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-

Approach	WB	SE	NW
HCM Control Delay, s	14.8	1.1	0
HCM LOS	B		

Minor Lane/Major Mvmt	NWT	NWRWBLn1	SEL	SET
Capacity (veh/h)	-	-	368	733
HCM Lane V/C Ratio	-	-	0.003	0.067
HCM Control Delay (s)	-	-	14.8	10.3
HCM Lane LOS	-	-	B	B
HCM 95th %tile Q(veh)	-	-	0	0.2

Intersection						
Int Delay, s/veh	2.3					
Movement	WBL	WBR	SEL	SET	NWT	NWR
Lane Configurations						
Traffic Vol, veh/h	0	116	51	337	651	91
Future Vol, veh/h	0	116	51	337	651	91
Conflicting Peds, #/hr	0	30	30	0	0	27
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	-	0	100	-	-	-
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	0	0	0	2	2	0
Mvmt Flow	0	126	55	366	708	99

Major/Minor	Minor2	Major1	Major2			
Conflicting Flow All	-	818	837	0	-	0
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Critical Hdwy	-	6.2	4.1	-	-	-
Critical Hdwy Stg 1	-	-	-	-	-	-
Critical Hdwy Stg 2	-	-	-	-	-	-
Follow-up Hdwy	-	3.3	2.2	-	-	-
Pot Cap-1 Maneuver	0	379	806	-	-	-
Stage 1	0	-	-	-	-	-
Stage 2	0	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	-	354	775	-	-	-
Mov Cap-2 Maneuver	-	-	-	-	-	-
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-

Approach	WB	SE	NW
HCM Control Delay, s	20.7	1.3	0
HCM LOS	C		













Minor Lane/Major Mvmt	NWT	NWRWBLn1	SEL	SET
Capacity (veh/h)	-	-	354	775
HCM Lane V/C Ratio	-	-	0.356	0.072
HCM Control Delay (s)	-	-	20.7	10
HCM Lane LOS	-	-	C	B
HCM 95th %tile Q(veh)	-	-	1.6	0.2

Intersection						
Int Delay, s/veh	0					
Movement	WBL	WBR	SEL	SET	NWT	NWR
Lane Configurations	Y			↑↑	↑	↑
Traffic Vol, veh/h	0	0	0	366	774	0
Future Vol, veh/h	0	0	0	366	774	0
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	100
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	0	0	0	398	841	0

Major/Minor	Minor2	Major1	Major2			
Conflicting Flow All	1040	841	841	0	-	0
Stage 1	841	-	-	-	-	-
Stage 2	199	-	-	-	-	-
Critical Hdwy	6.63	6.23	4.13	-	-	-
Critical Hdwy Stg 1	5.43	-	-	-	-	-
Critical Hdwy Stg 2	5.83	-	-	-	-	-
Follow-up Hdwy	3.519	3.319	2.219	-	-	-
Pot Cap-1 Maneuver	240	364	792	-	-	-
Stage 1	422	-	-	-	-	-
Stage 2	816	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	240	364	792	-	-	-
Mov Cap-2 Maneuver	240	-	-	-	-	-
Stage 1	422	-	-	-	-	-
Stage 2	816	-	-	-	-	-

Approach	WB	SE	NW
HCM Control Delay, s	0	0	0
HCM LOS	A		

Minor Lane/Major Mvmt	NWT	NWRWBLn1	SEL	SET
Capacity (veh/h)	-	-	-	792
HCM Lane V/C Ratio	-	-	-	-
HCM Control Delay (s)	-	-	0	0
HCM Lane LOS	-	-	A	A
HCM 95th %tile Q(veh)	-	-	-	0

						
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations						
Traffic Volume (vph)	860	303	227	616	281	631
Future Volume (vph)	860	303	227	616	281	631
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)		55	95		190	190
Storage Lanes		1	1		0	1
Taper Length (ft)			90		75	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor		0.91			0.92	0.96
Frt		0.850				0.850
Flt Protected			0.950		0.950	
Satd. Flow (prot)	1863	1583	1770	1863	1770	1583
Flt Permitted			0.080		0.950	
Satd. Flow (perm)	1863	1436	149	1863	1631	1519
Right Turn on Red		Yes				Yes
Satd. Flow (RTOR)		62				36
Link Speed (mph)	30			30	30	
Link Distance (ft)	390			893	363	
Travel Time (s)	8.9			20.3	8.3	
Confl. Peds. (#/hr)		50	50		50	50
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	935	329	247	670	305	686
Shared Lane Traffic (%)						
Lane Group Flow (vph)	935	329	247	670	305	686
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Left	Left	Right
Median Width(ft)	12			12	12	
Link Offset(ft)	0			0	0	
Crosswalk Width(ft)	16			16	16	
Two way Left Turn Lane						
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)		9	15		15	9
Number of Detectors	2	1	1	2	1	1
Detector Template	Thru	Right	Left	Thru	Left	Right
Leading Detector (ft)	100	20	20	100	20	20
Trailing Detector (ft)	0	0	0	0	0	0
Detector 1 Position(ft)	0	0	0	0	0	0
Detector 1 Size(ft)	6	20	20	6	20	20
Detector 1 Type	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex
Detector 1 Channel						
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0
Detector 2 Position(ft)	94			94		
Detector 2 Size(ft)	6			6		
Detector 2 Type	Cl+Ex			Cl+Ex		
Detector 2 Channel						
Detector 2 Extend (s)	0.0			0.0		
Turn Type	NA	Perm	pm+pt	NA	Prot	pt+ov



Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Protected Phases	4		3	8	5	5 3
Permitted Phases		4	8		5	5 3
Detector Phase	4	4	3	8	5	5 3
Switch Phase						
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	
Minimum Split (s)	25.0	25.0	9.5	25.0	10.0	
Total Split (s)	50.0	50.0	20.0	70.0	40.0	
Total Split (%)	45.5%	45.5%	18.2%	63.6%	36.4%	
Maximum Green (s)	45.0	45.0	17.0	65.0	35.0	
Yellow Time (s)	3.0	3.0	3.0	3.0	3.0	
All-Red Time (s)	2.0	2.0	0.0	2.0	2.0	
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	
Total Lost Time (s)	5.0	5.0	3.0	5.0	5.0	
Lead/Lag	Lag	Lag	Lead			
Lead-Lag Optimize?	Yes	Yes	Yes			
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	
Recall Mode	C-Min	C-Min	None	C-Min	Max	
Walk Time (s)	4.0	4.0		4.0		
Flash Dont Walk (s)	16.0	16.0		16.0		
Pedestrian Calls (#/hr)	0	0		0		
Act Effct Green (s)	47.2	47.2	67.0	65.0	35.0	52.8
Actuated g/C Ratio	0.43	0.43	0.61	0.59	0.32	0.48
v/c Ratio	1.17	0.51	0.80	0.61	0.54	0.88
Control Delay	108.0	16.3	44.2	17.4	31.7	38.3
Queue Delay	0.8	1.0	0.0	27.5	2.7	35.4
Total Delay	108.9	17.3	44.2	44.8	34.4	73.6
LOS	F	B	D	D	C	E
Approach Delay	85.0			44.7	61.5	
Approach LOS	F			D	E	
90th %ile Green (s)	45.0	45.0	17.0	65.0	35.0	
90th %ile Term Code	Coord	Coord	Max	Coord	MaxR	
70th %ile Green (s)	45.0	45.0	17.0	65.0	35.0	
70th %ile Term Code	Coord	Coord	Max	Coord	MaxR	
50th %ile Green (s)	45.0	45.0	17.0	65.0	35.0	
50th %ile Term Code	Coord	Coord	Max	Coord	MaxR	
30th %ile Green (s)	48.2	48.2	13.8	65.0	35.0	
30th %ile Term Code	Coord	Coord	Gap	Coord	MaxR	
10th %ile Green (s)	52.7	52.7	9.3	65.0	35.0	
10th %ile Term Code	Coord	Coord	Gap	Coord	MaxR	
Queue Length 50th (ft)	~810	86	113	284	168	457
Queue Length 95th (ft)	m#858	m94	#222	400	258	#639
Internal Link Dist (ft)	310			813	283	
Turn Bay Length (ft)		55	95		190	190
Base Capacity (vph)	799	651	341	1100	563	809
Starvation Cap Reductn	102	134	0	0	155	164
Spillback Cap Reductn	0	0	0	451	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	1.34	0.64	0.72	1.03	0.75	1.06

Intersection Summary

Area Type:	Other
Cycle Length:	110
Actuated Cycle Length:	110
Offset:	21 (19%), Referenced to phase 4:EBT and 8:WBTL, Start of Green
Natural Cycle:	90
Control Type:	Actuated-Coordinated
Maximum v/c Ratio:	1.17
Intersection Signal Delay:	66.0
Intersection LOS:	E
Intersection Capacity Utilization:	96.6%
ICU Level of Service:	F
Analysis Period (min):	15
~	Volume exceeds capacity, queue is theoretically infinite. Queue shown is maximum after two cycles.
#	95th percentile volume exceeds capacity, queue may be longer. Queue shown is maximum after two cycles.
m	Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 204: Jefferson St & Chicago Ave



Bally's Chicago Casino
305: Canal St/Jefferson St & Grand Ave

2026 Future with Entertainment District
Timing Plan: Saturday Casino PH



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	303	481	5	11	394	682	20	0	10	189	0	251
Future Volume (vph)	303	481	5	11	394	682	20	0	10	189	0	251
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	100		0	0		0	0		0	0		0
Storage Lanes	1		0	0		0	0		0	0		1
Taper Length (ft)	100			25			25			135		
Lane Util. Factor	1.00	0.95	0.95	0.95	0.95	0.95	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor		1.00			0.90			0.99			0.98	
Frt		0.999			0.906			0.955				0.850
Flt Protected	0.950				0.999			0.968			0.950	
Satd. Flow (prot)	1770	3534	0	0	2877	0	0	1705	0	0	1770	1583
Flt Permitted	0.114				0.948			0.968			0.950	
Satd. Flow (perm)	212	3534	0	0	2730	0	0	1705	0	0	1738	1583
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		1			421			109				273
Link Speed (mph)		30			30			30				30
Link Distance (ft)		375			838			269				430
Travel Time (s)		8.5			19.0			6.1				9.8
Confl. Peds. (#/hr)	56		10	10		56			13	13		
Confl. Bikes (#/hr)			6			10						
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	329	523	5	12	428	741	22	0	11	205	0	273
Shared Lane Traffic (%)												
Lane Group Flow (vph)	329	528	0	0	1181	0	0	33	0	0	205	273
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		12			0			0				0
Link Offset(ft)		0			0			0				0
Crosswalk Width(ft)		16			16			16				16
Two way Left Turn Lane												
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15		12	15		12	15		12	15		12
Number of Detectors	1	2		1	2		1	2		1	2	1
Detector Template	Left	Thru		Left	Thru		Left	Thru		Left	Thru	Right
Leading Detector (ft)	20	100		20	100		20	100		20	100	20
Trailing Detector (ft)	0	0		0	0		0	0		0	0	0
Detector 1 Position(ft)	0	0		0	0		0	0		0	0	0
Detector 1 Size(ft)	20	6		20	6		20	6		20	6	20
Detector 1 Type	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	Cl+Ex
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	0.0
Detector 2 Position(ft)		94			94			94				94
Detector 2 Size(ft)		6			6			6				6
Detector 2 Type		Cl+Ex			Cl+Ex			Cl+Ex				Cl+Ex
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0				0.0

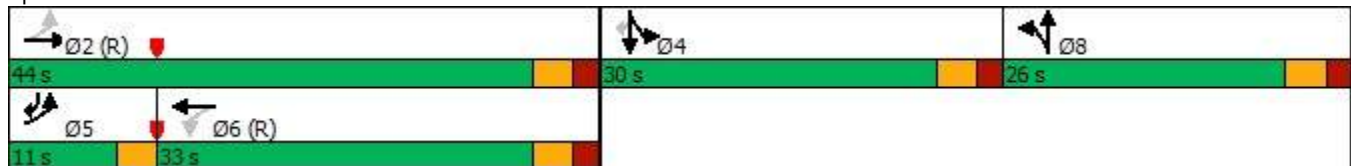


Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Turn Type	pm+pt	NA		Perm	NA		Split	NA		Split	NA	pm+ov
Protected Phases	5	2			6		8	8		4	4	5
Permitted Phases	2			6								4
Detector Phase	5	2		6	6		8	8		4	4	5
Switch Phase												
Minimum Initial (s)	5.0	5.0		5.0	5.0		5.0	5.0		5.0	5.0	5.0
Minimum Split (s)	10.0	30.0		30.0	30.0		26.0	26.0		30.0	30.0	10.0
Total Split (s)	11.0	44.0		33.0	33.0		26.0	26.0		30.0	30.0	11.0
Total Split (%)	11.0%	44.0%		33.0%	33.0%		26.0%	26.0%		30.0%	30.0%	11.0%
Maximum Green (s)	8.0	39.0		28.0	28.0		21.0	21.0		25.0	25.0	8.0
Yellow Time (s)	3.0	3.0		3.0	3.0		3.0	3.0		3.0	3.0	3.0
All-Red Time (s)	0.0	2.0		2.0	2.0		2.0	2.0		2.0	2.0	0.0
Lost Time Adjust (s)	0.0	0.0			0.0			0.0			0.0	0.0
Total Lost Time (s)	3.0	5.0			5.0			5.0			5.0	3.0
Lead/Lag	Lead			Lag	Lag							Lead
Lead-Lag Optimize?	Yes			Yes	Yes							Yes
Vehicle Extension (s)	3.0	3.0		3.0	3.0		3.0	3.0		3.0	3.0	3.0
Recall Mode	None	C-Max		C-Max	C-Max		None	None		None	None	None
Walk Time (s)		7.0		7.0	7.0		7.0	7.0				
Flash Dont Walk (s)		14.0		14.0	14.0		14.0	14.0				
Pedestrian Calls (#/hr)		0		0	0		0	0				
Act Effct Green (s)	68.9	66.9			34.3			5.5			16.8	49.6
Actuated g/C Ratio	0.69	0.67			0.34			0.06			0.17	0.50
v/c Ratio	0.54	0.22			0.97			0.17			0.69	0.30
Control Delay	13.8	6.5			42.7			1.9			50.9	1.9
Queue Delay	0.0	0.0			0.0			0.0			0.0	0.0
Total Delay	13.8	6.5			42.7			1.9			50.9	1.9
LOS	B	A			D			A			D	A
Approach Delay		9.3			42.7			1.9			22.9	
Approach LOS		A			D			A			C	
90th %ile Green (s)	25.7	56.7		28.0	28.0		5.5	5.5		22.8	22.8	25.7
90th %ile Term Code	Max	Coord		Coord	Coord		Gap	Gap		Gap	Gap	Max
70th %ile Green (s)	25.1	60.2		32.1	32.1		5.5	5.5		19.3	19.3	25.1
70th %ile Term Code	Gap	Coord		Coord	Coord		Gap	Gap		Gap	Gap	Gap
50th %ile Green (s)	25.4	62.7		34.3	34.3		5.5	5.5		16.8	16.8	25.4
50th %ile Term Code	Gap	Coord		Coord	Coord		Gap	Gap		Gap	Gap	Gap
30th %ile Green (s)	28.0	75.7		44.7	44.7		0.0	0.0		14.3	14.3	28.0
30th %ile Term Code	Gap	Coord		Coord	Coord		Skip	Skip		Gap	Gap	Gap
10th %ile Green (s)	44.1	79.3		32.2	32.2		0.0	0.0		10.7	10.7	44.1
10th %ile Term Code	Gap	Coord		Coord	Coord		Skip	Skip		Gap	Gap	Gap
Queue Length 50th (ft)	95	69			276			0			124	0
Queue Length 95th (ft)	155	96			#479			0			187	27
Internal Link Dist (ft)		295			758			189			350	
Turn Bay Length (ft)	100											
Base Capacity (vph)	607	2365			1212			444			442	923
Starvation Cap Reductn	0	0			0			0			0	0
Spillback Cap Reductn	0	0			0			0			0	0
Storage Cap Reductn	0	0			0			0			0	0
Reduced v/c Ratio	0.54	0.22			0.97			0.07			0.46	0.30

Intersection Summary

Area Type:	Other
Cycle Length:	100
Actuated Cycle Length:	100
Offset:	0 (0%), Referenced to phase 2:EBTL and 6:WBTL, Start of Green
Natural Cycle:	100
Control Type:	Actuated-Coordinated
Maximum v/c Ratio:	0.97
Intersection Signal Delay:	27.3
Intersection LOS:	C
Intersection Capacity Utilization	80.2%
ICU Level of Service	D
Analysis Period (min)	15
# 95th percentile volume exceeds capacity, queue may be longer. Queue shown is maximum after two cycles.	

Splits and Phases: 305: Canal St/Jefferson St & Grand Ave





Lane Group	WBL	WBR	NBT	NBR	SBL	SBT	Ø8
Lane Configurations							
Traffic Volume (vph)	256	496	415	438	256	274	
Future Volume (vph)	256	496	415	438	256	274	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	
Storage Length (ft)	0	0		160	205		
Storage Lanes	1	1		0	0		
Taper Length (ft)	25				70		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	
Ped Bike Factor	0.56	0.96		0.85			
Frt		0.850		0.850			
Flt Protected	0.950				0.950		
Satd. Flow (prot)	1805	1615	1863	646	1805	1453	
Flt Permitted	0.950				0.294		
Satd. Flow (perm)	1017	1546	1863	551	559	1453	
Right Turn on Red		Yes		Yes			
Satd. Flow (RTOR)		192		476			
Link Speed (mph)	30		30			30	
Link Distance (ft)	244		120			363	
Travel Time (s)	5.5		2.7			8.3	
Confl. Peds. (#/hr)	135	20		40	40		
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	
Heavy Vehicles (%)	0%	0%	2%	0%	0%	2%	
Parking (#/hr)				100		24	
Adj. Flow (vph)	278	539	451	476	278	298	
Shared Lane Traffic (%)							
Lane Group Flow (vph)	278	539	451	476	278	298	
Enter Blocked Intersection	No	No	No	No	No	No	
Lane Alignment	Left	Right	Left	Right	Left	Left	
Median Width(ft)	12		12			12	
Link Offset(ft)	0		0			0	
Crosswalk Width(ft)	16		16			16	
Two way Left Turn Lane							
Headway Factor	1.00	1.00	1.00	2.95	1.00	1.37	
Turning Speed (mph)	15	9		9	15		
Number of Detectors	1	1	2	1	1	2	
Detector Template	Left	Right	Thru	Right	Left	Thru	
Leading Detector (ft)	20	20	100	20	20	100	
Trailing Detector (ft)	0	0	0	0	0	0	
Detector 1 Position(ft)	0	0	0	0	0	0	
Detector 1 Size(ft)	20	20	6	20	20	6	
Detector 1 Type	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	
Detector 1 Channel							
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0	
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0	
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0	
Detector 2 Position(ft)			94			94	
Detector 2 Size(ft)			6			6	
Detector 2 Type			Cl+Ex			Cl+Ex	
Detector 2 Channel							



Lane Group	WBL	WBR	NBT	NBR	SBL	SBT	Ø8
Detector 2 Extend (s)			0.0			0.0	
Turn Type	custom	pt+ov	NA	custom	pm+pt	NA	
Protected Phases	7	17	2		1	6	8
Permitted Phases	8	7 8		2 8	6		
Detector Phase	7	17	2	2 8	1	6	
Switch Phase							
Minimum Initial (s)	5.0		5.0		5.0	5.0	5.0
Minimum Split (s)	16.0		22.0		16.0	23.0	22.0
Total Split (s)	22.0		49.0		16.0	65.0	23.0
Total Split (%)	20.0%		44.5%		14.5%	59.1%	21%
Maximum Green (s)	17.0		44.0		13.0	60.0	18.0
Yellow Time (s)	3.0		3.0		3.0	3.0	3.0
All-Red Time (s)	2.0		2.0		0.0	2.0	2.0
Lost Time Adjust (s)	0.0		0.0		0.0	0.0	
Total Lost Time (s)	5.0		5.0		3.0	5.0	
Lead/Lag	Lead		Lag		Lead		Lag
Lead-Lag Optimize?	Yes		Yes		Yes		Yes
Vehicle Extension (s)	3.0		3.0		3.0	3.0	3.0
Recall Mode	None		C-Max		None	C-Max	Max
Walk Time (s)			7.0			7.0	7.0
Flash Dont Walk (s)			10.0			11.0	10.0
Pedestrian Calls (#/hr)			25			25	25
Act Effct Green (s)	35.0	51.3	44.7	63.5	62.0	60.0	
Actuated g/C Ratio	0.32	0.47	0.41	0.58	0.56	0.55	
v/c Ratio	0.63	0.64	0.60	0.92	0.61	0.38	
Control Delay	34.0	15.1	29.8	27.8	8.6	4.1	
Queue Delay	0.0	2.6	5.3	0.0	0.0	0.5	
Total Delay	34.0	17.7	35.1	27.8	8.6	4.5	
LOS	C	B	D	C	A	A	
Approach Delay	23.3		31.3			6.5	
Approach LOS	C		C			A	
90th %ile Green (s)	17.0		44.0		13.0	60.0	18.0
90th %ile Term Code	Max		Coord		Max	Coord	MaxR
70th %ile Green (s)	17.0		44.0		13.0	60.0	18.0
70th %ile Term Code	Max		Coord		Max	Coord	MaxR
50th %ile Green (s)	17.0		44.0		13.0	60.0	18.0
50th %ile Term Code	Max		Coord		Max	Coord	MaxR
30th %ile Green (s)	17.0		44.6		12.4	60.0	18.0
30th %ile Term Code	Max		Coord		Gap	Coord	MaxR
10th %ile Green (s)	13.3		47.0		10.0	60.0	21.7
10th %ile Term Code	Gap		Coord		Gap	Coord	MaxR
Queue Length 50th (ft)	146	154	248	0	11	17	
Queue Length 95th (ft)	223	257	356	#143	m25	m19	
Internal Link Dist (ft)	164		40			283	
Turn Bay Length (ft)				160	205		
Base Capacity (vph)	452	859	757	519	462	792	
Starvation Cap Reductn	0	0	0	0	0	187	
Spillback Cap Reductn	0	204	239	0	0	0	
Storage Cap Reductn	0	0	0	0	0	0	

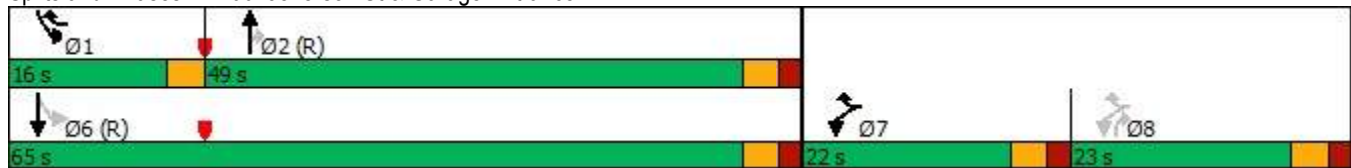


Lane Group	WBL	WBR	NBT	NBR	SBL	SBT	Ø8
Reduced v/c Ratio	0.62	0.82	0.87	0.92	0.60	0.49	

Intersection Summary

Area Type:	Other
Cycle Length:	110
Actuated Cycle Length:	110
Offset:	27 (25%), Referenced to phase 2:NBT and 6:SBTL, Start of Green
Natural Cycle:	90
Control Type:	Actuated-Coordinated
Maximum v/c Ratio:	0.92
Intersection Signal Delay:	22.3
Intersection LOS:	C
Intersection Capacity Utilization	62.0%
ICU Level of Service	B
Analysis Period (min)	15
#	95th percentile volume exceeds capacity, queue may be longer. Queue shown is maximum after two cycles.
m	Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 400: Jefferson St & Garage Entrance



Intersection						
Int Delay, s/veh	0.5					
Movement	WBL	WBR	SEL	SET	NWT	NWR
Lane Configurations		↗	↘	↑	↑	↗
Traffic Vol, veh/h	0	1	58	472	853	108
Future Vol, veh/h	0	1	58	472	853	108
Conflicting Peds, #/hr	0	0	40	0	0	68
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	-	0	100	-	-	50
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	0	2	2	0
Mvmt Flow	0	1	63	513	927	117

Major/Minor	Minor2	Major1	Major2			
Conflicting Flow All	-	995	1112	0	-	0
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Critical Hdwy	-	6.22	4.1	-	-	-
Critical Hdwy Stg 1	-	-	-	-	-	-
Critical Hdwy Stg 2	-	-	-	-	-	-
Follow-up Hdwy	-	3.318	2.2	-	-	-
Pot Cap-1 Maneuver	0	297	635	-	-	-
Stage 1	0	-	-	-	-	-
Stage 2	0	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	-	278	594	-	-	-
Mov Cap-2 Maneuver	-	-	-	-	-	-
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-

Approach	WB	SE	NW
HCM Control Delay, s	18	1.3	0
HCM LOS	C		

Minor Lane/Major Mvmt	NWT	NWRWBLn1	SEL	SET
Capacity (veh/h)	-	-	278	594
HCM Lane V/C Ratio	-	-	0.004	0.106
HCM Control Delay (s)	-	-	18	11.8
HCM Lane LOS	-	-	C	B
HCM 95th %tile Q(veh)	-	-	0	0.4

Intersection						
Int Delay, s/veh	3.8					
Movement	WBL	WBR	SEL	SET	NWT	NWR
Lane Configurations		↗	↖	↑	↘	
Traffic Vol, veh/h	0	139	64	405	829	116
Future Vol, veh/h	0	139	64	405	829	116
Conflicting Peds, #/hr	0	40	40	0	0	34
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	-	0	100	-	-	-
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	0	0	0	2	2	0
Mvmt Flow	0	151	70	440	901	126

Major/Minor	Minor2	Major1	Major2		
Conflicting Flow All	-	1044	1067	0	-
Stage 1	-	-	-	-	-
Stage 2	-	-	-	-	-
Critical Hdwy	-	6.2	4.1	-	-
Critical Hdwy Stg 1	-	-	-	-	-
Critical Hdwy Stg 2	-	-	-	-	-
Follow-up Hdwy	-	3.3	2.2	-	-
Pot Cap-1 Maneuver	0	281	661	-	-
Stage 1	0	-	-	-	-
Stage 2	0	-	-	-	-
Platoon blocked, %				-	-
Mov Cap-1 Maneuver	-	257	627	-	-
Mov Cap-2 Maneuver	-	-	-	-	-
Stage 1	-	-	-	-	-
Stage 2	-	-	-	-	-

Approach	WB	SE	NW
HCM Control Delay, s	37.2	1.6	0
HCM LOS	E		

Minor Lane/Major Mvmt	NWT	NWRWBLn1	SEL	SET
Capacity (veh/h)	-	-	257	627
HCM Lane V/C Ratio	-	-	0.588	0.111
HCM Control Delay (s)	-	-	37.2	11.5
HCM Lane LOS	-	-	E	B
HCM 95th %tile Q(veh)	-	-	3.4	0.4

Intersection						
Int Delay, s/veh	0					
Movement	WBL	WBR	SEL	SET	NWT	NWR
Lane Configurations						
Traffic Vol, veh/h	0	0	0	440	985	0
Future Vol, veh/h	0	0	0	440	985	0
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	100
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	0	0	0	478	1071	0

Major/Minor	Minor2	Major1	Major2			
Conflicting Flow All	1310	1071	1071	0	-	0
Stage 1	1071	-	-	-	-	-
Stage 2	239	-	-	-	-	-
Critical Hdwy	6.63	6.23	4.13	-	-	-
Critical Hdwy Stg 1	5.43	-	-	-	-	-
Critical Hdwy Stg 2	5.83	-	-	-	-	-
Follow-up Hdwy	3.519	3.319	2.219	-	-	-
Pot Cap-1 Maneuver	163	267	649	-	-	-
Stage 1	328	-	-	-	-	-
Stage 2	779	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	163	267	649	-	-	-
Mov Cap-2 Maneuver	163	-	-	-	-	-
Stage 1	328	-	-	-	-	-
Stage 2	779	-	-	-	-	-

Approach	WB	SE	NW
HCM Control Delay, s	0	0	0
HCM LOS	A		

Minor Lane/Major Mvmt	NWT	NWRWBLn1	SEL	SET
Capacity (veh/h)	-	-	-	649
HCM Lane V/C Ratio	-	-	-	-
HCM Control Delay (s)	-	-	0	0
HCM Lane LOS	-	-	A	A
HCM 95th %tile Q(veh)	-	-	-	0



























Capacity Analysis Worksheets

2032 Full Build-out PD 1426

Bally's Chicago Casino
204: Jefferson St & Chicago Ave

2032 Future with PD1426
Timing Plan: AM Peak Hour

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	121	1307	159	116	809	88	168	13	304	58	35	125
Future Volume (vph)	121	1307	159	116	809	88	168	13	304	58	35	125
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	80		55	95		55	190		190	0		0
Storage Lanes	1		1	1		1	0		1	1		0
Taper Length (ft)	100			90			75			25		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor			0.94			0.91	0.95		0.94	0.96	0.93	
Frt			0.850			0.850			0.850		0.883	
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1770	1863	1583	1770	1863	1583	1770	1863	1583	1770	1523	0
Flt Permitted	0.074			0.074			0.517			0.748		
Satd. Flow (perm)	138	1863	1493	138	1863	1433	912	1863	1493	1331	1523	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)			79			79			107			136
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		390			893			363			230	
Travel Time (s)		8.9			20.3			8.3			5.2	
Confl. Peds. (#/hr)	25		25	25		25	25		25	25		25
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	132	1421	173	126	879	96	183	14	330	63	38	136
Shared Lane Traffic (%)												
Lane Group Flow (vph)	132	1421	173	126	879	96	183	14	330	63	174	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		12			12			12			12	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15		9	15		9	15		9	15		9
Number of Detectors	1	2	1	1	2	1	1	2	1	1	2	
Detector Template	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	
Leading Detector (ft)	20	100	20	20	100	20	20	100	20	20	100	
Trailing Detector (ft)	0	0	0	0	0	0	0	0	0	0	0	
Detector 1 Position(ft)	0	0	0	0	0	0	0	0	0	0	0	
Detector 1 Size(ft)	20	6	20	20	6	20	20	6	20	20	6	
Detector 1 Type	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Detector 2 Position(ft)		94			94			94			94	
Detector 2 Size(ft)		6			6			6			6	
Detector 2 Type		Cl+Ex			Cl+Ex			Cl+Ex			Cl+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0			0.0	
Turn Type	pm+pt	NA	Perm	pm+pt	NA	Perm	pm+pt	NA	pm+ov	pm+pt	NA	

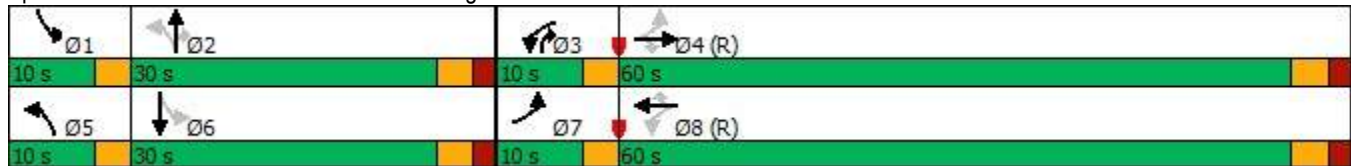


Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Protected Phases	7	4		3	8		5	2	3	1	6	
Permitted Phases	4		4	8		8	2		2	6		
Detector Phase	7	4	4	3	8	8	5	2	3	1	6	
Switch Phase												
Minimum Initial (s)	3.0	5.0	5.0	3.0	5.0	5.0	3.0	1.0	3.0	3.0	5.0	
Minimum Split (s)	8.0	58.0	58.0	8.0	58.0	58.0	8.0	28.0	8.0	8.0	28.0	
Total Split (s)	10.0	60.0	60.0	10.0	60.0	60.0	10.0	30.0	10.0	10.0	30.0	
Total Split (%)	9.1%	54.5%	54.5%	9.1%	54.5%	54.5%	9.1%	27.3%	9.1%	9.1%	27.3%	
Maximum Green (s)	7.0	55.0	55.0	7.0	55.0	55.0	7.0	25.0	7.0	7.0	25.0	
Yellow Time (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	
All-Red Time (s)	0.0	2.0	2.0	0.0	2.0	2.0	0.0	2.0	0.0	0.0	2.0	
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Lost Time (s)	3.0	5.0	5.0	3.0	5.0	5.0	3.0	5.0	3.0	3.0	5.0	
Lead/Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	Lead	Lead	Lag	
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	
Recall Mode	None	C-Max	C-Max	None	C-Max	C-Max	None	Max	None	None	Max	
Walk Time (s)		7.0	7.0		7.0	7.0		7.0			7.0	
Flash Dont Walk (s)		11.0	11.0		11.0	11.0		11.0			11.0	
Pedestrian Calls (#/hr)		0	0		0	0		0			0	
Act Effct Green (s)	64.0	55.0	55.0	64.0	55.0	55.0	34.6	27.0	36.0	33.7	25.0	
Actuated g/C Ratio	0.58	0.50	0.50	0.58	0.50	0.50	0.31	0.25	0.33	0.31	0.23	
v/c Ratio	0.72	1.53	0.22	0.68	0.94	0.13	0.54	0.03	0.58	0.14	0.39	
Control Delay	31.4	258.7	5.1	52.9	33.2	0.4	30.8	29.8	20.3	26.2	12.7	
Queue Delay	0.0	1.0	0.0	0.0	48.2	0.0	1.9	0.0	0.0	0.0	1.4	
Total Delay	31.4	259.7	5.1	52.9	81.3	0.4	32.7	29.8	20.4	26.2	14.1	
LOS	C	F	A	D	F	A	C	C	C	C	B	
Approach Delay		216.7			71.0			24.9			17.3	
Approach LOS		F			E			C			B	
90th %ile Green (s)	7.0	55.0	55.0	7.0	55.0	55.0	7.0	25.0	7.0	7.0	25.0	
90th %ile Term Code	Max	Coord	Coord	Max	Coord	Coord	Max	MaxR	Max	Max	MaxR	
70th %ile Green (s)	7.0	55.0	55.0	7.0	55.0	55.0	7.0	25.0	7.0	7.0	25.0	
70th %ile Term Code	Max	Coord	Coord	Max	Coord	Coord	Max	MaxR	Max	Max	MaxR	
50th %ile Green (s)	7.0	55.0	55.0	7.0	55.0	55.0	7.0	25.0	7.0	7.0	25.0	
50th %ile Term Code	Max	Coord	Coord	Max	Coord	Coord	Max	MaxR	Max	Max	MaxR	
30th %ile Green (s)	7.0	55.0	55.0	7.0	55.0	55.0	7.0	25.0	7.0	7.0	25.0	
30th %ile Term Code	Max	Coord	Coord	Max	Coord	Coord	Max	MaxR	Max	Max	MaxR	
10th %ile Green (s)	7.0	55.1	55.1	6.9	55.0	55.0	7.0	35.0	6.9	0.0	25.0	
10th %ile Term Code	Max	Coord	Coord	Gap	Coord	Coord	Max	MaxR	Gap	Skip	MaxR	
Queue Length 50th (ft)	52	~1459	13	38	630	0	81	7	83	30	21	
Queue Length 95th (ft)	m30	m#954	m5	m#111	#870	m0	135	22	158	62	81	
Internal Link Dist (ft)		310			813			283			150	
Turn Bay Length (ft)	80		55	95		55	190		190			
Base Capacity (vph)	184	931	786	184	931	756	341	457	566	439	451	
Starvation Cap Reductn	0	156	0	0	0	0	0	0	4	0	0	
Spillback Cap Reductn	0	0	0	0	483	0	64	0	0	0	139	
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	
Reduced v/c Ratio	0.72	1.83	0.22	0.68	1.96	0.13	0.66	0.03	0.59	0.14	0.56	

Intersection Summary

Area Type:	Other
Cycle Length:	110
Actuated Cycle Length:	110
Offset:	16 (15%), Referenced to phase 4:EBTL and 8:WBTL, Start of Green
Natural Cycle:	105
Control Type:	Actuated-Coordinated
Maximum v/c Ratio:	1.53
Intersection Signal Delay:	130.7
Intersection LOS:	F
Intersection Capacity Utilization	114.5%
ICU Level of Service	H
Analysis Period (min)	15
~ Volume exceeds capacity, queue is theoretically infinite. Queue shown is maximum after two cycles.	
# 95th percentile volume exceeds capacity, queue may be longer. Queue shown is maximum after two cycles.	
m Volume for 95th percentile queue is metered by upstream signal.	

Splits and Phases: 204: Jefferson St & Chicago Ave



Bally's Chicago Casino
305: Canal St/Jefferson St & Grand Ave

2032 Future with PD1426
Timing Plan: AM Peak Hour



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	168	838	3	11	556	351	28	0	26	118	0	200
Future Volume (vph)	168	838	3	11	556	351	28	0	26	118	0	200
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	100		0	0		0	0		0	0		0
Storage Lanes	1		0	0		0	0		0	0		1
Taper Length (ft)	100			25			25			135		
Lane Util. Factor	1.00	0.95	0.95	0.95	0.95	0.95	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor		1.00			0.90			0.97				0.95
Fr _t					0.943			0.935				0.850
Fl _t Protected	0.950				0.999			0.975			0.950	
Satd. Flow (prot)	1770	3538	0	0	2998	0	0	1649	0	0	1770	1583
Fl _t Permitted	0.200				0.941			0.975			0.950	
Satd. Flow (perm)	373	3538	0	0	2823	0	0	1649	0	0	1770	1508
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)					144			109				217
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		375			838			269			430	
Travel Time (s)		8.5			19.0			6.1			9.8	
Confl. Peds. (#/hr)	101		32	32		101			34			34
Confl. Bikes (#/hr)			6			9						
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	183	911	3	12	604	382	30	0	28	128	0	217
Shared Lane Traffic (%)												
Lane Group Flow (vph)	183	914	0	0	998	0	0	58	0	0	128	217
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		12			0			0			0	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15		9	15		9	15		9	15		9
Number of Detectors	1	2		1	2		1	2		1	2	1
Detector Template	Left	Thru		Left	Thru		Left	Thru		Left	Thru	Right
Leading Detector (ft)	20	100		20	100		20	100		20	100	20
Trailing Detector (ft)	0	0		0	0		0	0		0	0	0
Detector 1 Position(ft)	0	0		0	0		0	0		0	0	0
Detector 1 Size(ft)	20	6		20	6		20	6		20	6	20
Detector 1 Type	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	Cl+Ex
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	0.0
Detector 2 Position(ft)		94			94			94			94	
Detector 2 Size(ft)		6			6			6			6	
Detector 2 Type		Cl+Ex			Cl+Ex			Cl+Ex			Cl+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0			0.0	

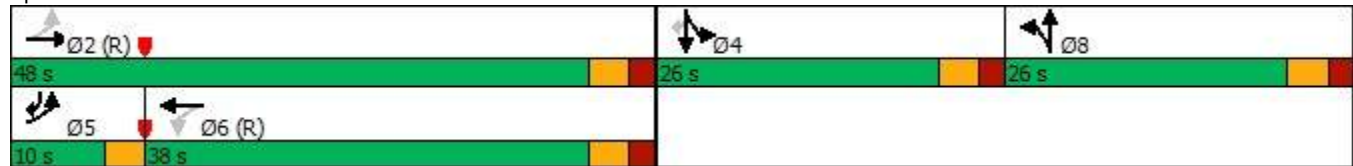


Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Turn Type	pm+pt	NA		Perm	NA		Split	NA		Split	NA	pm+ov
Protected Phases	5	2			6		8	8		4	4	5
Permitted Phases	2			6								4
Detector Phase	5	2		6	6		8	8		4	4	5
Switch Phase												
Minimum Initial (s)	5.0	5.0		5.0	5.0		5.0	5.0		5.0	5.0	5.0
Minimum Split (s)	10.0	26.0		26.0	26.0		26.0	26.0		23.0	23.0	10.0
Total Split (s)	10.0	48.0		38.0	38.0		26.0	26.0		26.0	26.0	10.0
Total Split (%)	10.0%	48.0%		38.0%	38.0%		26.0%	26.0%		26.0%	26.0%	10.0%
Maximum Green (s)	7.0	43.0		33.0	33.0		21.0	21.0		21.0	21.0	7.0
Yellow Time (s)	3.0	3.0		3.0	3.0		3.0	3.0		3.0	3.0	3.0
All-Red Time (s)	0.0	2.0		2.0	2.0		2.0	2.0		2.0	2.0	0.0
Lost Time Adjust (s)	0.0	0.0			0.0			0.0			0.0	0.0
Total Lost Time (s)	3.0	5.0			5.0			5.0			5.0	3.0
Lead/Lag	Lead			Lag	Lag							Lead
Lead-Lag Optimize?	Yes			Yes	Yes							Yes
Vehicle Extension (s)	3.0	3.0		3.0	3.0		3.0	3.0		3.0	3.0	3.0
Recall Mode	None	C-Max		C-Max	C-Max		None	None		None	None	None
Walk Time (s)		7.0		7.0	7.0		7.0	7.0				
Flash Dont Walk (s)		14.0		14.0	14.0		14.0	14.0				
Pedestrian Calls (#/hr)		0		0	0		0	0				
Act Effct Green (s)	71.0	69.0			54.4			5.6			12.5	26.1
Actuated g/C Ratio	0.71	0.69			0.54			0.06			0.12	0.26
v/c Ratio	0.43	0.37			0.62			0.30			0.58	0.39
Control Delay	9.0	7.9			17.1			5.1			51.2	4.4
Queue Delay	0.0	0.7			0.0			0.0			0.0	0.0
Total Delay	9.0	8.6			17.1			5.1			51.2	4.4
LOS	A	A			B			A			D	A
Approach Delay		8.7			17.1			5.1			21.7	
Approach LOS		A			B			A			C	
90th %ile Green (s)	15.1	61.7		43.6	43.6		6.0	6.0		17.3	17.3	15.1
90th %ile Term Code	Gap	Coord		Coord	Coord		Gap	Gap		Gap	Gap	Gap
70th %ile Green (s)	12.3	65.0		49.7	49.7		5.5	5.5		14.5	14.5	12.3
70th %ile Term Code	Gap	Coord		Coord	Coord		Gap	Gap		Gap	Gap	Gap
50th %ile Green (s)	11.3	67.0		52.7	52.7		5.5	5.5		12.5	12.5	11.3
50th %ile Term Code	Gap	Coord		Coord	Coord		Gap	Gap		Gap	Gap	Gap
30th %ile Green (s)	10.7	69.0		55.3	55.3		5.5	5.5		10.5	10.5	10.7
30th %ile Term Code	Gap	Coord		Coord	Coord		Gap	Gap		Gap	Gap	Gap
10th %ile Green (s)	8.8	82.3		70.5	70.5		0.0	0.0		7.7	7.7	8.8
10th %ile Term Code	Gap	Coord		Coord	Coord		Skip	Skip		Gap	Gap	Gap
Queue Length 50th (ft)	35	121			194			0			78	0
Queue Length 95th (ft)	72	186			322			7			131	37
Internal Link Dist (ft)		295			758			189			350	
Turn Bay Length (ft)	100											
Base Capacity (vph)	427	2441			1600			432			371	563
Starvation Cap Reductn	0	1093			0			0			0	0
Spillback Cap Reductn	0	0			0			0			0	0
Storage Cap Reductn	0	0			0			0			0	0
Reduced v/c Ratio	0.43	0.68			0.62			0.13			0.35	0.39

Intersection Summary

Area Type:	Other
Cycle Length:	100
Actuated Cycle Length:	100
Offset:	0 (0%), Referenced to phase 2:EBTL and 6:WBTL, Start of Green
Natural Cycle:	85
Control Type:	Actuated-Coordinated
Maximum v/c Ratio:	0.62
Intersection Signal Delay:	13.8
Intersection LOS:	B
Intersection Capacity Utilization	80.5%
ICU Level of Service	D
Analysis Period (min)	15

Splits and Phases: 305: Canal St/Jefferson St & Grand Ave



Bally's Chicago Casino
400: Jefferson St & Garage Entrance

2032 Future with PD1426
Timing Plan: AM Peak Hour

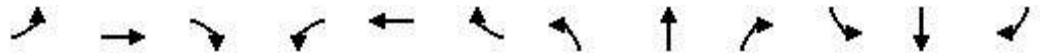


Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↔			↔	↔	↔	↑	↔	↔	↔	↔
Traffic Volume (vph)	52	0	29	103	0	202	23	229	202	118	165	27
Future Volume (vph)	52	0	29	103	0	202	23	229	202	118	165	27
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0		0	0		0	60		100	205		0
Storage Lanes	0		0	0		1	1		0	0		0
Taper Length (ft)	25			25			55			70		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor		0.95			0.86	0.95	0.98		0.95	0.98	0.99	
Frt		0.951				0.850			0.850		0.979	
Flt Protected		0.969			0.950		0.950			0.950		
Satd. Flow (prot)	0	1701	0	0	1805	1615	1805	1863	775	1805	1816	0
Flt Permitted		0.758			0.712		0.628			0.563		
Satd. Flow (perm)	0	1304	0	0	1157	1538	1166	1863	736	1050	1816	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		50				220			220			18
Link Speed (mph)		30			30			30				30
Link Distance (ft)		130			244			120				363
Travel Time (s)		3.0			5.5			2.7				8.3
Confl. Peds. (#/hr)	20		20	59		20	10		10	10		10
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	0%	0%	0%	0%	0%	0%	0%	2%	0%	0%	2%	0%
Parking (#/hr)									84			
Adj. Flow (vph)	57	0	32	112	0	220	25	249	220	128	179	29
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	89	0	0	112	220	25	249	220	128	208	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		0			0			12			12	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	2.41	1.00	1.00	1.00
Turning Speed (mph)	15		9	15		9	15		9	15		9
Number of Detectors	1	2		1	2	1	1	2	1	1		2
Detector Template	Left	Thru		Left	Thru	Right	Left	Thru	Right	Left	Thru	
Leading Detector (ft)	20	100		20	100	20	20	100	20	20	100	
Trailing Detector (ft)	0	0		0	0	0	0	0	0	0	0	
Detector 1 Position(ft)	0	0		0	0	0	0	0	0	0	0	
Detector 1 Size(ft)	20	6		20	6	20	20	6	20	20	6	
Detector 1 Type	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Detector 1 Queue (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Detector 1 Delay (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Detector 2 Position(ft)		94			94			94			94	
Detector 2 Size(ft)		6			6			6			6	
Detector 2 Type		Cl+Ex			Cl+Ex			Cl+Ex			Cl+Ex	
Detector 2 Channel												

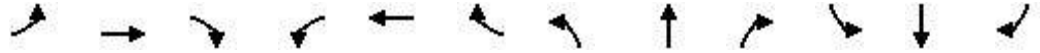
Bally's Chicago Casino
400: Jefferson St & Garage Entrance

2032 Future with PD1426

Timing Plan: AM Peak Hour



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Detector 2 Extend (s)		0.0			0.0			0.0			0.0	
Turn Type	Perm	NA		Perm	NA	pm+ov	Perm	NA	Perm	pm+pt	NA	
Protected Phases		4			8	1		2		1	6	
Permitted Phases	4			8		8	2		2	6		
Detector Phase	4	4		8	8	1	2	2	2	1	6	
Switch Phase												
Minimum Initial (s)	5.0	5.0		5.0	5.0	5.0	1.0	1.0	1.0	5.0	5.0	
Minimum Split (s)	23.0	23.0		23.0	23.0	16.0	23.0	23.0	23.0	16.0	23.0	
Total Split (s)	27.0	27.0		27.0	27.0	18.0	65.0	65.0	65.0	18.0	83.0	
Total Split (%)	24.5%	24.5%		24.5%	24.5%	16.4%	59.1%	59.1%	59.1%	16.4%	75.5%	
Maximum Green (s)	22.0	22.0		22.0	22.0	15.0	60.0	60.0	60.0	15.0	78.0	
Yellow Time (s)	3.0	3.0		3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	
All-Red Time (s)	2.0	2.0		2.0	2.0	0.0	2.0	2.0	2.0	0.0	2.0	
Lost Time Adjust (s)		0.0			0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Lost Time (s)		5.0			5.0	3.0	5.0	5.0	5.0	3.0	5.0	
Lead/Lag						Lead	Lag	Lag	Lag	Lead		
Lead-Lag Optimize?						Yes	Yes	Yes	Yes	Yes		
Vehicle Extension (s)	3.0	3.0		3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	
Recall Mode	None	None		Max	Max	None	C-Max	C-Max	C-Max	None	C-Max	
Walk Time (s)	7.0	7.0		7.0	7.0		7.0	7.0	7.0		7.0	
Flash Dont Walk (s)	11.0	11.0		11.0	11.0		11.0	11.0	11.0		11.0	
Pedestrian Calls (#/hr)	0	0		0	0		0	0	0		0	
Act Effct Green (s)		22.0			22.0	31.8	67.2	67.2	67.2	80.0	78.0	
Actuated g/C Ratio		0.20			0.20	0.29	0.61	0.61	0.61	0.73	0.71	
v/c Ratio		0.30			0.48	0.36	0.04	0.22	0.41	0.16	0.16	
Control Delay		21.5			47.1	5.4	9.1	10.4	3.8	6.7	7.3	
Queue Delay		0.0			0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Delay		21.5			47.1	5.4	9.1	10.4	3.8	6.7	7.3	
LOS		C			D	A	A	B	A	A	A	
Approach Delay		21.5			19.5			7.4			7.1	
Approach LOS		C			B			A			A	
90th %ile Green (s)	22.0	22.0		22.0	22.0	9.4	65.6	65.6	65.6	9.4	78.0	
90th %ile Term Code	Hold	Hold		MaxR	MaxR	Gap	Coord	Coord	Coord	Gap	Coord	
70th %ile Green (s)	22.0	22.0		22.0	22.0	8.4	66.6	66.6	66.6	8.4	78.0	
70th %ile Term Code	Hold	Hold		MaxR	MaxR	Gap	Coord	Coord	Coord	Gap	Coord	
50th %ile Green (s)	22.0	22.0		22.0	22.0	7.7	67.3	67.3	67.3	7.7	78.0	
50th %ile Term Code	Hold	Hold		MaxR	MaxR	Gap	Coord	Coord	Coord	Gap	Coord	
30th %ile Green (s)	22.0	22.0		22.0	22.0	7.1	67.9	67.9	67.9	7.1	78.0	
30th %ile Term Code	Hold	Hold		MaxR	MaxR	Gap	Coord	Coord	Coord	Gap	Coord	
10th %ile Green (s)	22.0	22.0		22.0	22.0	6.2	68.8	68.8	68.8	6.2	78.0	
10th %ile Term Code	Hold	Hold		MaxR	MaxR	Gap	Coord	Coord	Coord	Gap	Coord	
Queue Length 50th (ft)		23			71	0	7	73	0	35	61	
Queue Length 95th (ft)		69			130	52	18	118	34	m51	m81	
Internal Link Dist (ft)		50			164			40			283	
Turn Bay Length (ft)							60		100	205		
Base Capacity (vph)		300			231	697	712	1138	535	866	1292	
Starvation Cap Reductn		0			0	0	0	0	0	0	0	
Spillback Cap Reductn		0			0	0	0	0	0	0	0	
Storage Cap Reductn		0			0	0	0	0	0	0	0	



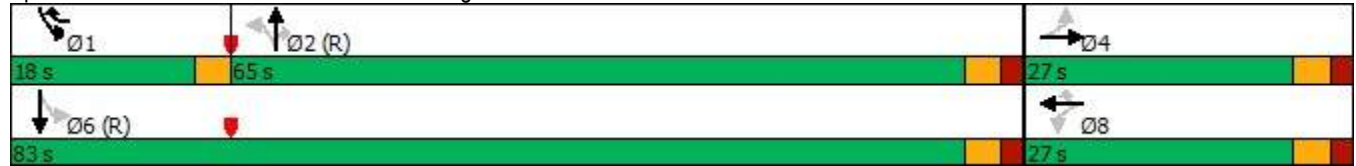
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Reduced v/c Ratio		0.30			0.48	0.32	0.04	0.22	0.41	0.15	0.16	

Intersection Summary

Area Type:	Other
Cycle Length:	110
Actuated Cycle Length:	110
Offset:	94 (85%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green
Natural Cycle:	65
Control Type:	Actuated-Coordinated
Maximum v/c Ratio:	0.48
Intersection Signal Delay:	11.5
Intersection LOS:	B
Intersection Capacity Utilization	51.1%
ICU Level of Service	A
Analysis Period (min)	15

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 400: Jefferson St & Garage Entrance



Intersection												
Int Delay, s/veh	1											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	SEL	SET	SER	NWL	NWT	NWR
Lane Configurations		↕				↕	↕	↕		↕	↕	↕
Traffic Vol, veh/h	19	1	15	0	0	0	27	256	14	8	435	48
Future Vol, veh/h	19	1	15	0	0	0	27	256	14	8	435	48
Conflicting Peds, #/hr	0	0	0	0	0	0	10	0	0	0	0	30
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	0	100	-	-	50	-	50
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	92	92	92	92	92	92	92	92	92	92	92	92
Heavy Vehicles, %	0	0	0	2	2	2	0	2	2	2	2	0
Mvmt Flow	21	1	16	0	0	0	29	278	15	9	473	52

Major/Minor	Minor1		Minor2		Major1		Major2					
Conflicting Flow All	861	917	286	-	-	503	555	0	0	293	0	0
Stage 1	344	344	-	-	-	-	-	-	-	-	-	-
Stage 2	517	573	-	-	-	-	-	-	-	-	-	-
Critical Hdwy	7.1	6.5	6.2	-	-	6.22	4.1	-	-	4.12	-	-
Critical Hdwy Stg 1	6.1	5.5	-	-	-	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.1	5.5	-	-	-	-	-	-	-	-	-	-
Follow-up Hdwy	3.5	4	3.3	-	-	3.318	2.2	-	-	2.218	-	-
Pot Cap-1 Maneuver	278	274	758	0	0	569	1026	-	-	1269	-	-
Stage 1	676	640	-	0	0	-	-	-	-	-	-	-
Stage 2	545	507	-	0	0	-	-	-	-	-	-	-
Platoon blocked, %								-	-	-	-	-
Mov Cap-1 Maneuver	270	256	758	-	-	553	997	-	-	1269	-	-
Mov Cap-2 Maneuver	270	256	-	-	-	-	-	-	-	-	-	-
Stage 1	656	621	-	-	-	-	-	-	-	-	-	-
Stage 2	541	489	-	-	-	-	-	-	-	-	-	-

Approach	EB	WB	SE	NW
HCM Control Delay, s	15.8	0	0.8	0.1
HCM LOS	C	A		

Minor Lane/Major Mvmt	NWL	NWT	NWR	EBLn1WBLn1	SEL	SET	SER
Capacity (veh/h)	1269	-	-	372	-	997	-
HCM Lane V/C Ratio	0.007	-	-	0.102	-	0.029	-
HCM Control Delay (s)	7.9	-	-	15.8	0	8.7	-
HCM Lane LOS	A	-	-	C	A	A	-
HCM 95th %tile Q(veh)	0	-	-	0.3	-	0.1	-

Intersection												
Int Delay, s/veh	1.9											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	SEL	SET	SER	NWL	NWT	NWR
Lane Configurations		↕					↗	↖	↔		↗	↖
Traffic Vol, veh/h	14	0	21	0	0	57	29	237	2	14	424	52
Future Vol, veh/h	14	0	21	0	0	57	29	237	2	14	424	52
Conflicting Peds, #/hr	20	0	20	0	0	10	10	0	20	20	0	15
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	0	100	-	-	50	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	92	92	92	92	92	92	92	92	92	92	92	92
Heavy Vehicles, %	2	0	2	0	0	0	0	2	2	2	2	0
Mvmt Flow	15	0	23	0	0	62	32	258	2	15	461	57

Major/Minor	Minor1		Minor2		Major1		Major2					
Conflicting Flow All	914	906	299	-	-	525	533	0	0	280	0	0
Stage 1	343	343	-	-	-	-	-	-	-	-	-	-
Stage 2	571	563	-	-	-	-	-	-	-	-	-	-
Critical Hdwy	7.12	6.5	6.22	-	-	6.2	4.1	-	-	4.12	-	-
Critical Hdwy Stg 1	6.12	5.5	-	-	-	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.12	5.5	-	-	-	-	-	-	-	-	-	-
Follow-up Hdwy	3.518	4	3.318	-	-	3.3	2.2	-	-	2.218	-	-
Pot Cap-1 Maneuver	254	278	741	0	0	556	1045	-	-	1283	-	-
Stage 1	672	641	-	0	0	-	-	-	-	-	-	-
Stage 2	506	512	-	0	0	-	-	-	-	-	-	-
Platoon blocked, %								-	-	-	-	-
Mov Cap-1 Maneuver	209	256	713	-	-	535	1025	-	-	1259	-	-
Mov Cap-2 Maneuver	209	256	-	-	-	-	-	-	-	-	-	-
Stage 1	638	610	-	-	-	-	-	-	-	-	-	-
Stage 2	434	496	-	-	-	-	-	-	-	-	-	-

Approach	EB		WB		SE		NW	
HCM Control Delay, s	16.1		12.6		0.9		0.2	
HCM LOS	C		B					

Minor Lane/Major Mvmt	NWL	NWT	NWR	EBLn1	WBLn1	SEL	SET	SER
Capacity (veh/h)	1259	-	-	363	535	1025	-	-
HCM Lane V/C Ratio	0.012	-	-	0.105	0.116	0.031	-	-
HCM Control Delay (s)	7.9	-	-	16.1	12.6	8.6	-	-
HCM Lane LOS	A	-	-	C	B	A	-	-
HCM 95th %tile Q(veh)	0	-	-	0.3	0.4	0.1	-	-

Intersection												
Int Delay, s/veh	2.3											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	SEL	SET	SER	NWL	NWT	NWR
Lane Configurations		↕			↕			↕			↕	↕
Traffic Vol, veh/h	7	23	0	53	0	21	2	259	11	1	481	19
Future Vol, veh/h	7	23	0	53	0	21	2	259	11	1	481	19
Conflicting Peds, #/hr	0	0	0	20	0	20	20	0	0	0	0	20
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	100
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	92	92	92	92	92	92	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	8	25	0	58	0	23	2	282	12	1	523	21

























Major/Minor	Minor1		Minor2		Major1			Major2				
Conflicting Flow All	859	858	167	723	843	563	564	0	0	294	0	0
Stage 1	292	292	-	545	545	-	-	-	-	-	-	-
Stage 2	567	566	-	178	298	-	-	-	-	-	-	-
Critical Hdwy	7.33	6.53	6.93	7.33	6.53	6.23	4.13	-	-	4.13	-	-
Critical Hdwy Stg 1	6.53	5.53	-	6.13	5.53	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.13	5.53	-	6.53	5.53	-	-	-	-	-	-	-
Follow-up Hdwy	3.519	4.019	3.319	3.519	4.019	3.319	2.219	-	-	2.219	-	-
Pot Cap-1 Maneuver	263	294	849	327	300	525	1006	-	-	1266	-	-
Stage 1	692	670	-	522	518	-	-	-	-	-	-	-
Stage 2	507	507	-	807	666	-	-	-	-	-	-	-
Platoon blocked, %								-	-	-	-	-
Mov Cap-1 Maneuver	246	288	833	293	293	505	987	-	-	1266	-	-
Mov Cap-2 Maneuver	246	288	-	293	293	-	-	-	-	-	-	-
Stage 1	691	669	-	511	508	-	-	-	-	-	-	-
Stage 2	474	497	-	761	665	-	-	-	-	-	-	-

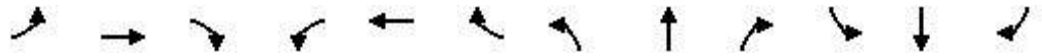
Approach	EB		WB		SE		NW	
HCM Control Delay, s	19.7		19.2		0.1		0	
HCM LOS	C		C					

Minor Lane/Major Mvmt	NWL	NWT	NWR	EBLn1	WBLn1	SEL	SET	SER
Capacity (veh/h)	1266	-	-	277	333	987	-	-
HCM Lane V/C Ratio	0.001	-	-	0.118	0.242	0.002	-	-
HCM Control Delay (s)	7.8	0	-	19.7	19.2	8.7	0	-
HCM Lane LOS	A	A	-	C	C	A	A	-
HCM 95th %tile Q(veh)	0	-	-	0.4	0.9	0	-	-

Bally's Chicago Casino
204: Jefferson St & Chicago Ave

2032 Future with PD1426
Timing Plan: PM Peak Hour

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	85	1087	206	157	1330	62	221	9	445	90	54	177
Future Volume (vph)	85	1087	206	157	1330	62	221	9	445	90	54	177
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	80		55	95		55	190		190	0		0
Storage Lanes	1		1	1		1	0		1	1		0
Taper Length (ft)	100			90			75			25		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor			0.90			0.83	0.92		0.90	0.91	0.87	
Frt			0.850			0.850			0.850		0.885	
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1770	1863	1583	1770	1863	1583	1770	1863	1583	1770	1435	0
Flt Permitted	0.082			0.077			0.416			0.751		
Satd. Flow (perm)	153	1863	1422	143	1863	1315	709	1863	1422	1272	1435	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)			79			79			216			148
Link Speed (mph)		30			30			30				30
Link Distance (ft)		390			893			363				230
Travel Time (s)		8.9			20.3			8.3				5.2
Confl. Peds. (#/hr)	50		50	50		50	50		50	50		50
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	92	1182	224	171	1446	67	240	10	484	98	59	192
Shared Lane Traffic (%)												
Lane Group Flow (vph)	92	1182	224	171	1446	67	240	10	484	98	251	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		12			12			12				12
Link Offset(ft)		0			0			0				0
Crosswalk Width(ft)		16			16			16				16
Two way Left Turn Lane												
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	60		60	60		60	60		60	60		60
Number of Detectors	1	2	1	1	2	1	1	2	1	1	2	
Detector Template	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	
Leading Detector (ft)	20	100	20	20	100	20	20	100	20	20	100	
Trailing Detector (ft)	0	0	0	0	0	0	0	0	0	0	0	
Detector 1 Position(ft)	0	0	0	0	0	0	0	0	0	0	0	
Detector 1 Size(ft)	20	6	20	20	6	20	20	6	20	20	6	
Detector 1 Type	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 2 Position(ft)		94			94			94				94
Detector 2 Size(ft)		6			6			6				6
Detector 2 Type		Cl+Ex			Cl+Ex			Cl+Ex				Cl+Ex
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0				0.0
Turn Type	pm+pt	NA	Perm	pm+pt	NA	Perm	pm+pt	NA	Perm	pm+pt	NA	

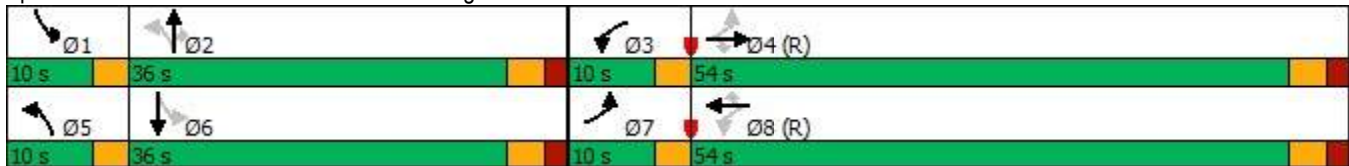


Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Protected Phases	7	4		3	8		5	2		1	6	
Permitted Phases	4		4	8		8	2		2	6		
Detector Phase	7	4	4	3	8	8	5	2	2	1	6	
Switch Phase												
Minimum Initial (s)	3.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	3.0	5.0	
Minimum Split (s)	8.0	42.0	42.0	8.0	50.0	50.0	8.0	35.0	35.0	8.0	25.0	
Total Split (s)	10.0	54.0	54.0	10.0	54.0	54.0	10.0	36.0	36.0	10.0	36.0	
Total Split (%)	9.1%	49.1%	49.1%	9.1%	49.1%	49.1%	9.1%	32.7%	32.7%	9.1%	32.7%	
Maximum Green (s)	7.0	49.0	49.0	7.0	49.0	49.0	7.0	31.0	31.0	7.0	31.0	
Yellow Time (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	
All-Red Time (s)	0.0	2.0	2.0	0.0	2.0	2.0	0.0	2.0	2.0	0.0	2.0	
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Lost Time (s)	3.0	5.0	5.0	3.0	5.0	5.0	3.0	5.0	5.0	3.0	5.0	
Lead/Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes						
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	
Recall Mode	None	C-Max	C-Max	Max	C-Max	C-Max	Max	None	None	None	None	
Walk Time (s)		7.0	7.0		7.0	7.0		7.0	7.0		7.0	
Flash Dont Walk (s)		11.0	11.0		11.0	11.0		11.0	11.0		11.0	
Pedestrian Calls (#/hr)		0	0		0	0		0	0		0	
Act Effct Green (s)	57.8	49.0	49.0	62.9	54.7	54.7	36.4	27.4	27.4	36.2	27.3	
Actuated g/C Ratio	0.53	0.45	0.45	0.57	0.50	0.50	0.33	0.25	0.25	0.33	0.25	
v/c Ratio	0.51	1.43	0.33	0.72	1.56	0.10	0.79	0.02	0.94	0.22	0.54	
Control Delay	23.1	226.2	14.0	29.2	280.0	3.7	41.8	25.0	42.0	24.0	18.2	
Queue Delay	0.0	1.1	0.0	0.0	0.0	0.0	0.0	0.0	1.0	0.0	0.0	
Total Delay	23.1	227.3	14.0	29.2	280.0	3.7	41.8	25.0	43.0	24.0	18.2	
LOS	C	F	B	C	F	A	D	C	D	C	B	
Approach Delay		182.9			243.5			42.4			19.8	
Approach LOS		F			F			D			B	
90th %ile Green (s)	7.0	49.0	49.0	7.0	49.0	49.0	7.0	31.0	31.0	7.0	31.0	
90th %ile Term Code	Max	Coord	Coord	MaxR	Coord	Coord	MaxR	Max	Max	Max	Hold	
70th %ile Green (s)	7.0	49.0	49.0	7.0	49.0	49.0	7.0	31.0	31.0	7.0	31.0	
70th %ile Term Code	Max	Coord	Coord	MaxR	Coord	Coord	MaxR	Max	Max	Max	Hold	
50th %ile Green (s)	7.0	49.0	49.0	7.0	49.0	49.0	7.0	31.0	31.0	7.0	31.0	
50th %ile Term Code	Max	Coord	Coord	MaxR	Coord	Coord	MaxR	Max	Max	Max	Hold	
30th %ile Green (s)	7.1	49.0	49.0	11.8	53.7	53.7	7.0	26.2	26.2	7.0	26.2	
30th %ile Term Code	Gap	Coord	Coord	MaxR	Coord	Coord	MaxR	Gap	Gap	Max	Hold	
10th %ile Green (s)	0.0	49.0	49.0	20.7	72.7	72.7	7.0	17.7	17.7	6.6	17.3	
10th %ile Term Code	Skip	Coord	Coord	MaxR	Coord	Coord	MaxR	Gap	Gap	Gap	Hold	
Queue Length 50th (ft)	29	~1130	62	57	~1559	10	86	4	81	44	56	
Queue Length 95th (ft)	62	#1385	120	m#88	m#1656	m12	#157	m11	#390	81	135	
Internal Link Dist (ft)		310			813			283			150	
Turn Bay Length (ft)	80		55	95		55	190		190			
Base Capacity (vph)	183	829	677	239	925	693	302	525	555	451	510	
Starvation Cap Reductn	0	139	0	0	0	0	0	0	11	0	0	
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	
Reduced v/c Ratio	0.50	1.71	0.33	0.72	1.56	0.10	0.79	0.02	0.89	0.22	0.49	

Intersection Summary

Area Type:	Other
Cycle Length:	110
Actuated Cycle Length:	110
Offset:	13 (12%), Referenced to phase 4:EBTL and 8:WBTL, Start of Green
Natural Cycle:	105
Control Type:	Actuated-Coordinated
Maximum v/c Ratio:	1.56
Intersection Signal Delay:	169.3
Intersection LOS:	F
Intersection Capacity Utilization:	118.7%
ICU Level of Service:	H
Analysis Period (min):	15
~ Volume exceeds capacity, queue is theoretically infinite. Queue shown is maximum after two cycles.	
# 95th percentile volume exceeds capacity, queue may be longer. Queue shown is maximum after two cycles.	
m Volume for 95th percentile queue is metered by upstream signal.	

Splits and Phases: 204: Jefferson St & Chicago Ave



Bally's Chicago Casino
305: Canal St/Jefferson St & Grand Ave

2032 Future with PD1426
Timing Plan: PM Peak Hour



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	206	714	7	15	712	470	22	0	14	158	0	268
Future Volume (vph)	206	714	7	15	712	470	22	0	14	158	0	268
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	100		0	0		0	0		0	0		0
Storage Lanes	1		0	0		0	0		0	0		1
Taper Length (ft)	100			25			25			135		
Lane Util. Factor	1.00	0.95	0.95	0.95	0.95	0.95	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor		1.00			0.85			0.97			0.93	
Frt		0.998			0.941			0.948				0.850
Flt Protected	0.950				0.999			0.970			0.950	
Satd. Flow (prot)	1770	3528	0	0	2818	0	0	1660	0	0	1770	1583
Flt Permitted	0.082				0.941			0.970			0.950	
Satd. Flow (perm)	153	3528	0	0	2654	0	0	1660	0	0	1652	1583
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		1			158			109				291
Link Speed (mph)		30			30			30				30
Link Distance (ft)		375			838			269				430
Travel Time (s)		8.5			19.0			6.1				9.8
Confl. Peds. (#/hr)	149		35	35		149			49	49		
Confl. Bikes (#/hr)			2			30						
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	224	776	8	16	774	511	24	0	15	172	0	291
Shared Lane Traffic (%)												
Lane Group Flow (vph)	224	784	0	0	1301	0	0	39	0	0	172	291
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		12			0			0				0
Link Offset(ft)		0			0			0				0
Crosswalk Width(ft)		16			16			16				16
Two way Left Turn Lane												
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15		9	15		9	15		9	15		9
Number of Detectors	1	2		1	2		1	2		1	2	1
Detector Template	Left	Thru		Left	Thru		Left	Thru		Left	Thru	Right
Leading Detector (ft)	20	100		20	100		20	100		20	100	20
Trailing Detector (ft)	0	0		0	0		0	0		0	0	0
Detector 1 Position(ft)	0	0		0	0		0	0		0	0	0
Detector 1 Size(ft)	20	6		20	6		20	6		20	6	20
Detector 1 Type	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	Cl+Ex
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	0.0
Detector 2 Position(ft)		94			94			94				94
Detector 2 Size(ft)		6			6			6				6
Detector 2 Type		Cl+Ex			Cl+Ex			Cl+Ex				Cl+Ex
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0				0.0

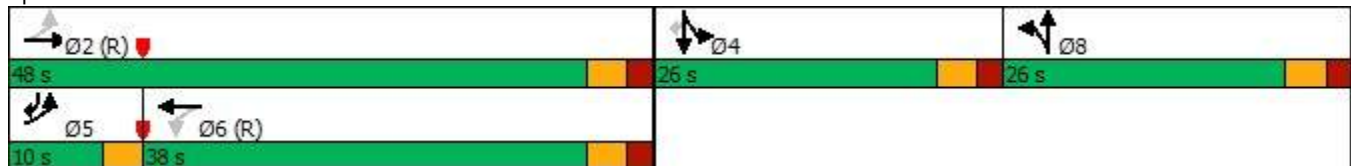


Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Turn Type	pm+pt	NA		Perm	NA		Split	NA		Split	NA	pm+ov
Protected Phases	5	2			6		8	8		4	4	5
Permitted Phases	2			6								4
Detector Phase	5	2		6	6		8	8		4	4	5
Switch Phase												
Minimum Initial (s)	5.0	5.0		5.0	5.0		5.0	5.0		5.0	5.0	5.0
Minimum Split (s)	10.0	26.0		26.0	26.0		26.0	26.0		23.0	23.0	10.0
Total Split (s)	10.0	48.0		38.0	38.0		26.0	26.0		26.0	26.0	10.0
Total Split (%)	10.0%	48.0%		38.0%	38.0%		26.0%	26.0%		26.0%	26.0%	10.0%
Maximum Green (s)	7.0	43.0		33.0	33.0		21.0	21.0		21.0	21.0	7.0
Yellow Time (s)	3.0	3.0		3.0	3.0		3.0	3.0		3.0	3.0	3.0
All-Red Time (s)	0.0	2.0		2.0	2.0		2.0	2.0		2.0	2.0	0.0
Lost Time Adjust (s)	0.0	0.0			0.0			0.0			0.0	0.0
Total Lost Time (s)	3.0	5.0			5.0			5.0			5.0	3.0
Lead/Lag	Lead			Lag	Lag							Lead
Lead-Lag Optimize?	Yes			Yes	Yes							Yes
Vehicle Extension (s)	3.0	3.0		3.0	3.0		3.0	3.0		3.0	3.0	3.0
Recall Mode	None	C-Max		C-Max	C-Max		None	None		None	None	None
Walk Time (s)		7.0		7.0	7.0		7.0	7.0				
Flash Dont Walk (s)		14.0		14.0	14.0		14.0	14.0				
Pedestrian Calls (#/hr)		0		0	0		0	0				
Act Effct Green (s)	70.8	68.8			48.8			5.5			14.9	35.1
Actuated g/C Ratio	0.71	0.69			0.49			0.06			0.15	0.35
v/c Ratio	0.59	0.32			0.94			0.20			0.65	0.39
Control Delay	24.2	7.9			38.4			2.3			51.2	3.2
Queue Delay	0.0	0.5			0.0			0.0			0.0	0.0
Total Delay	24.2	8.4			38.4			2.3			51.2	3.2
LOS	C	A			D			A			D	A
Approach Delay		11.9			38.4			2.3			21.0	
Approach LOS		B			D			A			C	
90th %ile Green (s)	19.4	59.1		36.7	36.7		5.5	5.5		20.4	20.4	19.4
90th %ile Term Code	Gap	Coord		Coord	Coord		Gap	Gap		Gap	Gap	Gap
70th %ile Green (s)	16.2	62.3		43.1	43.1		5.5	5.5		17.2	17.2	16.2
70th %ile Term Code	Gap	Coord		Coord	Coord		Gap	Gap		Gap	Gap	Gap
50th %ile Green (s)	15.3	64.5		46.2	46.2		5.5	5.5		15.0	15.0	15.3
50th %ile Term Code	Gap	Coord		Coord	Coord		Gap	Gap		Gap	Gap	Gap
30th %ile Green (s)	14.9	77.3		59.4	59.4		0.0	0.0		12.7	12.7	14.9
30th %ile Term Code	Gap	Coord		Coord	Coord		Skip	Skip		Gap	Gap	Gap
10th %ile Green (s)	18.8	80.6		58.8	58.8		0.0	0.0		9.4	9.4	18.8
10th %ile Term Code	Gap	Coord		Coord	Coord		Skip	Skip		Gap	Gap	Gap
Queue Length 50th (ft)	78	108			385			0			105	0
Queue Length 95th (ft)	159	166			#639			0			163	36
Internal Link Dist (ft)		295			758			189			350	
Turn Bay Length (ft)	100											
Base Capacity (vph)	382	2426			1377			434			371	744
Starvation Cap Reductn	0	1079			0			0			0	0
Spillback Cap Reductn	0	0			0			0			0	0
Storage Cap Reductn	0	0			0			0			0	0
Reduced v/c Ratio	0.59	0.58			0.94			0.09			0.46	0.39

Intersection Summary

Area Type:	Other
Cycle Length:	100
Actuated Cycle Length:	100
Offset:	0 (0%), Referenced to phase 2:EBTL and 6:WBTL, Start of Green
Natural Cycle:	85
Control Type:	Actuated-Coordinated
Maximum v/c Ratio:	0.94
Intersection Signal Delay:	25.5
Intersection LOS:	C
Intersection Capacity Utilization:	86.5%
ICU Level of Service:	E
Analysis Period (min)	15
# 95th percentile volume exceeds capacity, queue may be longer. Queue shown is maximum after two cycles.	

Splits and Phases: 305: Canal St/Jefferson St & Grand Ave



Bally's Chicago Casino
400: Jefferson St & Garage Entrance

2032 Future with PD1426
Timing Plan: PM Peak Hour

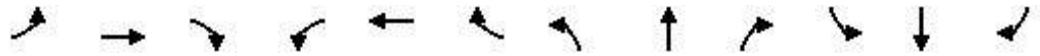


Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↔			↔	↔	↔	↑	↔	↔	↔	↔
Traffic Volume (vph)	37	0	21	170	0	329	26	308	257	149	237	31
Future Volume (vph)	37	0	21	170	0	329	26	308	257	149	237	31
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0		0	0		0	60		100	205		0
Storage Lanes	0		0	0		1	1		0	0		0
Taper Length (ft)	25			25			55			70		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor		0.96			0.77	0.95	0.96		0.92	0.97	0.99	
Frt		0.951				0.850			0.850		0.983	
Flt Protected		0.969			0.950		0.950			0.950		
Satd. Flow (prot)	0	1700	0	0	1805	1615	1805	1863	646	1805	1818	0
Flt Permitted		0.764			0.748		0.581			0.466		
Satd. Flow (perm)	0	1319	0	0	1094	1538	1059	1863	594	862	1818	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		50				358			279			12
Link Speed (mph)		30			30			30				30
Link Distance (ft)		130			244			120				363
Travel Time (s)		3.0			5.5			2.7				8.3
Confl. Peds. (#/hr)	20		20	84		20	20		20	20		20
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	0%	0%	0%	0%	0%	0%	0%	2%	0%	0%	2%	0%
Parking (#/hr)									100			
Adj. Flow (vph)	40	0	23	185	0	358	28	335	279	162	258	34
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	63	0	0	185	358	28	335	279	162	292	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		0			0			12			12	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	2.95	1.00	1.00	1.00
Turning Speed (mph)	60		60	15		9	60		9	15		60
Number of Detectors	1	2		1	2	1	1	2	1	1		2
Detector Template	Left	Thru		Left	Thru	Right	Left	Thru	Right	Left	Thru	
Leading Detector (ft)	20	100		20	100	20	20	100	20	20	100	
Trailing Detector (ft)	0	0		0	0	0	0	0	0	0	0	
Detector 1 Position(ft)	0	0		0	0	0	0	0	0	0	0	
Detector 1 Size(ft)	20	6		20	6	20	20	6	20	20	6	
Detector 1 Type	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Detector 1 Queue (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Detector 1 Delay (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Detector 2 Position(ft)		94			94			94			94	
Detector 2 Size(ft)		6			6			6			6	
Detector 2 Type		Cl+Ex			Cl+Ex			Cl+Ex			Cl+Ex	
Detector 2 Channel												

Bally's Chicago Casino
400: Jefferson St & Garage Entrance

2032 Future with PD1426

Timing Plan: PM Peak Hour



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Detector 2 Extend (s)		0.0			0.0			0.0			0.0	
Turn Type	Perm	NA		Perm	NA	pm+ov	Perm	NA	Perm	pm+pt	NA	
Protected Phases		4			8	1		2		1	6	
Permitted Phases	4			8		8	2		2	6		
Detector Phase	4	4		8	8	1	2	2	2	1	6	
Switch Phase												
Minimum Initial (s)	5.0	5.0		5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	
Minimum Split (s)	23.0	23.0		23.0	23.0	16.0	23.0	23.0	23.0	16.0	23.0	
Total Split (s)	35.0	35.0		35.0	35.0	17.0	58.0	58.0	58.0	17.0	75.0	
Total Split (%)	31.8%	31.8%		31.8%	31.8%	15.5%	52.7%	52.7%	52.7%	15.5%	68.2%	
Maximum Green (s)	30.0	30.0		30.0	30.0	14.0	53.0	53.0	53.0	14.0	70.0	
Yellow Time (s)	3.0	3.0		3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	
All-Red Time (s)	2.0	2.0		2.0	2.0	0.0	2.0	2.0	2.0	0.0	2.0	
Lost Time Adjust (s)		0.0			0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Lost Time (s)		5.0			5.0	3.0	5.0	5.0	5.0	3.0	5.0	
Lead/Lag						Lead	Lag	Lag	Lag	Lead		
Lead-Lag Optimize?						Yes	Yes	Yes	Yes	Yes		
Vehicle Extension (s)	3.0	3.0		3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	
Recall Mode	None	None		Max	Max	None	C-Max	C-Max	C-Max	None	C-Max	
Walk Time (s)	7.0	7.0		7.0	7.0		7.0	7.0	7.0		7.0	
Flash Dont Walk (s)	11.0	11.0		11.0	11.0		11.0	11.0	11.0		11.0	
Pedestrian Calls (#/hr)	0	0		0	0		0	0	0		0	
Act Effct Green (s)		30.0			30.0	41.5	57.5	57.5	57.5	72.0	70.0	
Actuated g/C Ratio		0.27			0.27	0.38	0.52	0.52	0.52	0.65	0.64	
v/c Ratio		0.16			0.62	0.44	0.05	0.34	0.63	0.25	0.25	
Control Delay		12.5			45.5	3.9	14.1	16.9	9.2	12.0	14.8	
Queue Delay		0.0			0.0	0.0	0.0	0.0	0.0	0.0	1.2	
Total Delay		12.5			45.5	4.0	14.1	16.9	9.2	12.0	16.0	
LOS		B			D	A	B	B	A	B	B	
Approach Delay		12.5			18.1			13.5			14.6	
Approach LOS		B			B			B			B	
90th %ile Green (s)	30.0	30.0		30.0	30.0	12.7	54.3	54.3	54.3	12.7	70.0	
90th %ile Term Code	Hold	Hold		MaxR	MaxR	Gap	Coord	Coord	Coord	Gap	Coord	
70th %ile Green (s)	30.0	30.0		30.0	30.0	10.2	56.8	56.8	56.8	10.2	70.0	
70th %ile Term Code	Hold	Hold		MaxR	MaxR	Gap	Coord	Coord	Coord	Gap	Coord	
50th %ile Green (s)	30.0	30.0		30.0	30.0	9.2	57.8	57.8	57.8	9.2	70.0	
50th %ile Term Code	Hold	Hold		MaxR	MaxR	Gap	Coord	Coord	Coord	Gap	Coord	
30th %ile Green (s)	30.0	30.0		30.0	30.0	8.3	58.7	58.7	58.7	8.3	70.0	
30th %ile Term Code	Hold	Hold		MaxR	MaxR	Gap	Coord	Coord	Coord	Gap	Coord	
10th %ile Green (s)	30.0	30.0		30.0	30.0	7.0	60.0	60.0	60.0	7.0	70.0	
10th %ile Term Code	Hold	Hold		MaxR	MaxR	Gap	Coord	Coord	Coord	Gap	Coord	
Queue Length 50th (ft)		7			114	0	9	131	0	49	109	
Queue Length 95th (ft)		40			195	53	26	209	81	m88	173	
Internal Link Dist (ft)		50			164			40			283	
Turn Bay Length (ft)							60		100	205		
Base Capacity (vph)		396			298	861	553	974	443	684	1161	
Starvation Cap Reductn		0			0	0	0	0	0	0	644	
Spillback Cap Reductn		0			0	4	0	25	0	0	0	
Storage Cap Reductn		0			0	0	0	0	0	0	0	

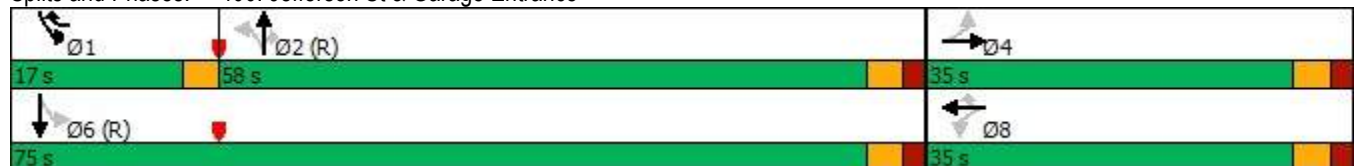


Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Reduced v/c Ratio		0.16			0.62	0.42	0.05	0.35	0.63	0.24	0.56	

Intersection Summary

Area Type: Other
 Cycle Length: 110
 Actuated Cycle Length: 110
 Offset: 79 (72%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green
 Natural Cycle: 65
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.63
 Intersection Signal Delay: 15.2
 Intersection LOS: B
 Intersection Capacity Utilization 59.6%
 ICU Level of Service B
 Analysis Period (min) 15
 m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 400: Jefferson St & Garage Entrance



Intersection												
Int Delay, s/veh	1.1											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	SEL	SET	SER	NWL	NWT	NWR
Lane Configurations		↕				↕	↕	↕		↕	↕	↕
Traffic Vol, veh/h	19	2	13	0	0	0	34	380	14	14	572	62
Future Vol, veh/h	19	2	13	0	0	0	34	380	14	14	572	62
Conflicting Peds, #/hr	0	0	0	0	0	0	20	0	0	0	0	42
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	0	100	-	-	50	-	50
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	92	92	92	92	92	92	92	92	92	92	92	92
Heavy Vehicles, %	0	0	0	2	2	2	0	2	2	2	2	0
Mvmt Flow	21	2	14	0	0	0	37	413	15	15	622	67

Major/Minor	Minor1		Minor2		Major1		Major2					
Conflicting Flow All	1181	1256	421	-	-	664	731	0	0	428	0	0
Stage 1	495	495	-	-	-	-	-	-	-	-	-	-
Stage 2	686	761	-	-	-	-	-	-	-	-	-	-
Critical Hdwy	7.1	6.5	6.2	-	-	6.22	4.1	-	-	4.12	-	-
Critical Hdwy Stg 1	6.1	5.5	-	-	-	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.1	5.5	-	-	-	-	-	-	-	-	-	-
Follow-up Hdwy	3.5	4	3.3	-	-	3.318	2.2	-	-	2.218	-	-
Pot Cap-1 Maneuver	168	173	637	0	0	461	883	-	-	1131	-	-
Stage 1	560	549	-	0	0	-	-	-	-	-	-	-
Stage 2	441	417	-	0	0	-	-	-	-	-	-	-
Platoon blocked, %								-	-	-	-	-
Mov Cap-1 Maneuver	161	157	637	-	-	443	848	-	-	1131	-	-
Mov Cap-2 Maneuver	161	157	-	-	-	-	-	-	-	-	-	-
Stage 1	535	525	-	-	-	-	-	-	-	-	-	-
Stage 2	435	395	-	-	-	-	-	-	-	-	-	-

Approach	EB	WB	SE	NW
HCM Control Delay, s	24.1	0	0.7	0.2
HCM LOS	C	A		

Minor Lane/Major Mvmt	NWL	NWT	NWR	EBLn1WBLn1	SEL	SET	SER
Capacity (veh/h)	1131	-	-	225	-	848	-
HCM Lane V/C Ratio	0.013	-	-	0.164	-	0.044	-
HCM Control Delay (s)	8.2	-	-	24.1	0	9.4	-
HCM Lane LOS	A	-	-	C	A	A	-
HCM 95th %tile Q(veh)	0	-	-	0.6	-	0.1	-

Intersection												
Int Delay, s/veh	2.4											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	SEL	SET	SER	NWL	NWT	NWR
Lane Configurations		↕				↕	↕	↕		↕	↕	
Traffic Vol, veh/h	14	0	18	0	0	91	39	350	4	25	544	69
Future Vol, veh/h	14	0	18	0	0	91	39	350	4	25	544	69
Conflicting Peds, #/hr	20	0	20	0	0	20	20	0	20	20	0	21
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	0	100	-	-	50	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	92	92	92	92	92	92	92	92	92	92	92	92
Heavy Vehicles, %	2	0	2	0	0	0	0	2	2	2	2	0
Mvmt Flow	15	0	20	0	0	99	42	380	4	27	591	75

Major/Minor	Minor1		Minor2		Major1		Major2					
Conflicting Flow All	1238	1227	422	-	-	670	687	0	0	404	0	0
Stage 1	486	486	-	-	-	-	-	-	-	-	-	-
Stage 2	752	741	-	-	-	-	-	-	-	-	-	-
Critical Hdwy	7.12	6.5	6.22	-	-	6.2	4.1	-	-	4.12	-	-
Critical Hdwy Stg 1	6.12	5.5	-	-	-	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.12	5.5	-	-	-	-	-	-	-	-	-	-
Follow-up Hdwy	3.518	4	3.318	-	-	3.3	2.2	-	-	2.218	-	-
Pot Cap-1 Maneuver	152	180	632	0	0	460	916	-	-	1155	-	-
Stage 1	563	554	-	0	0	-	-	-	-	-	-	-
Stage 2	402	426	-	0	0	-	-	-	-	-	-	-
Platoon blocked, %								-	-	-	-	-
Mov Cap-1 Maneuver	107	160	608	-	-	439	892	-	-	1133	-	-
Mov Cap-2 Maneuver	107	160	-	-	-	-	-	-	-	-	-	-
Stage 1	526	518	-	-	-	-	-	-	-	-	-	-
Stage 2	298	405	-	-	-	-	-	-	-	-	-	-

Approach	EB		WB		SE		NW	
HCM Control Delay, s	26.9		15.6		0.9		0.3	
HCM LOS	D		C					

Minor Lane/Major Mvmt	NWL	NWT	NWR	EBLn1	WBLn1	SEL	SET	SER
Capacity (veh/h)	1133	-	-	199	439	892	-	-
HCM Lane V/C Ratio	0.024	-	-	0.175	0.225	0.048	-	-
HCM Control Delay (s)	8.3	-	-	26.9	15.6	9.2	-	-
HCM Lane LOS	A	-	-	D	C	A	-	-
HCM 95th %tile Q(veh)	0.1	-	-	0.6	0.9	0.1	-	-

Intersection												
Int Delay, s/veh	3.1											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	SEL	SET	SER	NWL	NWT	NWR
Lane Configurations		↕			↕			↕			↕	↕
Traffic Vol, veh/h	9	40	0	45	0	19	4	369	18	1	634	33
Future Vol, veh/h	9	40	0	45	0	19	4	369	18	1	634	33
Conflicting Peds, #/hr	0	0	0	20	0	20	20	0	0	0	0	20
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	100
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	92	92	92	92	92	92	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	10	43	0	49	0	21	4	401	20	1	689	36

Major/Minor	Minor1		Minor2		Major1			Major2				
Conflicting Flow All	1159	1166	231	961	1140	729	745	0	0	421	0	0
Stage 1	419	419	-	711	711	-	-	-	-	-	-	-
Stage 2	740	747	-	250	429	-	-	-	-	-	-	-
Critical Hdwy	7.33	6.53	6.93	7.33	6.53	6.23	4.13	-	-	4.13	-	-
Critical Hdwy Stg 1	6.53	5.53	-	6.13	5.53	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.13	5.53	-	6.53	5.53	-	-	-	-	-	-	-
Follow-up Hdwy	3.519	4.019	3.319	3.519	4.019	3.319	2.219	-	-	2.219	-	-
Pot Cap-1 Maneuver	161	193	772	223	200	422	861	-	-	1136	-	-
Stage 1	583	589	-	423	435	-	-	-	-	-	-	-
Stage 2	408	419	-	733	583	-	-	-	-	-	-	-
Platoon blocked, %								-	-	-	-	-
Mov Cap-1 Maneuver	149	188	757	176	195	406	845	-	-	1136	-	-
Mov Cap-2 Maneuver	149	188	-	176	195	-	-	-	-	-	-	-
Stage 1	580	585	-	412	426	-	-	-	-	-	-	-
Stage 2	379	411	-	662	580	-	-	-	-	-	-	-

Approach	EB		WB		SE		NW	
HCM Control Delay, s	33.4		30.1		0.1		0	
HCM LOS	D		D					

Minor Lane/Major Mvmt	NWL	NWT	NWR	EBLn1	WBLn1	SEL	SET	SER
Capacity (veh/h)	1136	-	-	179	212	845	-	-
HCM Lane V/C Ratio	0.001	-	-	0.298	0.328	0.005	-	-
HCM Control Delay (s)	8.2	0	-	33.4	30.1	9.3	0	-
HCM Lane LOS	A	A	-	D	D	A	A	-
HCM 95th %tile Q(veh)	0	-	-	1.2	1.4	0	-	-

Bally's Chicago Casino
204: Jefferson St & Chicago Ave

2032 Future with PD1426
Timing Plan: Friday Casino PH

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	17	811	251	190	577	12	254	2	544	18	11	35
Future Volume (vph)	17	811	251	190	577	12	254	2	544	18	11	35
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	80		55	95		55	190		190	0		0
Storage Lanes	1		1	1		1	0		1	1		0
Taper Length (ft)	100			90			75			25		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor	0.97		0.90			0.83	0.87		0.90	0.91	0.87	
Frt			0.850			0.850			0.850		0.886	
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1770	1863	1583	1770	1863	1583	1770	1863	1583	1770	1438	0
Flt Permitted	0.262			0.090			0.619			0.757		
Satd. Flow (perm)	472	1863	1422	168	1863	1315	1002	1863	1422	1281	1438	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)			139			109			384			38
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		390			893			363			230	
Travel Time (s)		8.9			20.3			8.3			5.2	
Confl. Peds. (#/hr)	50		50	50		50	50		50	50		50
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	18	882	273	207	627	13	276	2	591	20	12	38
Shared Lane Traffic (%)												
Lane Group Flow (vph)	18	882	273	207	627	13	276	2	591	20	50	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		12			12			12			12	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	60		60	60		60	60		60	60		60
Number of Detectors	1	2	1	1	2	1	1	2	1	1	2	
Detector Template	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	
Leading Detector (ft)	20	100	20	20	100	20	20	100	20	20	100	
Trailing Detector (ft)	0	0	0	0	0	0	0	0	0	0	0	
Detector 1 Position(ft)	0	0	0	0	0	0	0	0	0	0	0	
Detector 1 Size(ft)	20	6	20	20	6	20	20	6	20	20	6	
Detector 1 Type	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Detector 2 Position(ft)		94			94			94			94	
Detector 2 Size(ft)		6			6			6			6	
Detector 2 Type		Cl+Ex			Cl+Ex			Cl+Ex			Cl+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0			0.0	
Turn Type	pm+pt	NA	Perm	pm+pt	NA	Perm	pm+pt	NA	Perm	pm+pt	NA	

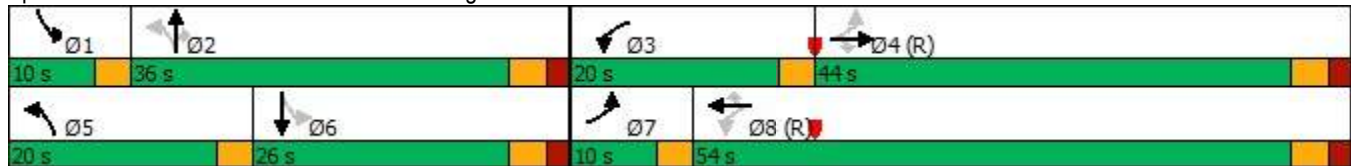


Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Protected Phases	7	4		3	8		5	2		1	6	
Permitted Phases	4		4	8		8	2		2	6		
Detector Phase	7	4	4	3	8	8	5	2	2	1	6	
Switch Phase												
Minimum Initial (s)	3.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	3.0	3.0	
Minimum Split (s)	8.0	44.0	44.0	16.0	54.0	54.0	16.0	35.0	35.0	8.0	25.0	
Total Split (s)	10.0	44.0	44.0	20.0	54.0	54.0	20.0	36.0	36.0	10.0	26.0	
Total Split (%)	9.1%	40.0%	40.0%	18.2%	49.1%	49.1%	18.2%	32.7%	32.7%	9.1%	23.6%	
Maximum Green (s)	7.0	39.0	39.0	17.0	49.0	49.0	17.0	31.0	31.0	7.0	21.0	
Yellow Time (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	
All-Red Time (s)	0.0	2.0	2.0	0.0	2.0	2.0	0.0	2.0	2.0	0.0	2.0	
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Lost Time (s)	3.0	5.0	5.0	3.0	5.0	5.0	3.0	5.0	5.0	3.0	5.0	
Lead/Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	
Recall Mode	None	C-Max	C-Max	None	C-Max	C-Max	Max	Max	Max	None	None	
Walk Time (s)		7.0	7.0		7.0	7.0		7.0	7.0		7.0	
Flash Dont Walk (s)		11.0	11.0		11.0	11.0		11.0	11.0		11.0	
Pedestrian Calls (#/hr)		0	0		0	0		0	0		0	
Act Effct Green (s)	51.1	43.1	43.1	61.0	55.2	55.2	43.0	37.1	37.1	24.6	17.9	
Actuated g/C Ratio	0.46	0.39	0.39	0.55	0.50	0.50	0.39	0.34	0.34	0.22	0.16	
v/c Ratio	0.06	1.21	0.43	0.74	0.67	0.02	0.51	0.00	0.81	0.06	0.19	
Control Delay	12.0	132.1	13.3	37.0	26.2	0.1	25.4	26.0	21.1	22.1	17.7	
Queue Delay	0.0	0.7	0.4	0.0	5.3	0.0	0.9	0.0	1.2	0.0	0.0	
Total Delay	12.0	132.8	13.8	37.0	31.4	0.1	26.3	26.0	22.4	22.1	17.7	
LOS	B	F	B	D	C	A	C	C	C	C	B	
Approach Delay		103.2			32.3			23.6			18.9	
Approach LOS		F			C			C			B	
90th %ile Green (s)	6.8	39.0	39.0	17.0	49.2	49.2	17.0	31.0	31.0	7.0	21.0	
90th %ile Term Code	Gap	Coord	Coord	Max	Coord	Coord	MaxR	MaxR	MaxR	Max	Hold	
70th %ile Green (s)	6.3	40.2	40.2	15.8	49.7	49.7	17.0	31.3	31.3	6.7	21.0	
70th %ile Term Code	Gap	Coord	Coord	Gap	Coord	Coord	MaxR	MaxR	MaxR	Gap	Hold	
50th %ile Green (s)	0.0	43.1	43.1	12.9	59.0	59.0	17.0	41.0	41.0	0.0	21.0	
50th %ile Term Code	Skip	Coord	Coord	Gap	Coord	Coord	MaxR	MaxR	MaxR	Skip	Hold	
30th %ile Green (s)	0.0	45.5	45.5	10.5	59.0	59.0	17.0	41.0	41.0	0.0	21.0	
30th %ile Term Code	Skip	Coord	Coord	Gap	Coord	Coord	MaxR	MaxR	MaxR	Skip	Hold	
10th %ile Green (s)	0.0	47.6	47.6	8.4	59.0	59.0	43.0	41.0	41.0	0.0	0.0	
10th %ile Term Code	Skip	Coord	Coord	Gap	Coord	Coord	Hold	MaxR	MaxR	Skip	Skip	
Queue Length 50th (ft)	5	~751	44	85	295	0	147	1	168	9	7	
Queue Length 95th (ft)	m8	m#1027	m109	163	508	0	207	m2	#374	25	41	
Internal Link Dist (ft)		310			813			283			150	
Turn Bay Length (ft)	80		55	95		55	190		190			
Base Capacity (vph)	306	729	641	340	934	713	546	627	733	326	305	
Starvation Cap Reductn	0	70	105	0	0	0	96	0	39	0	0	
Spillback Cap Reductn	0	0	0	0	241	0	0	0	0	0	4	
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	
Reduced v/c Ratio	0.06	1.34	0.51	0.61	0.90	0.02	0.61	0.00	0.85	0.06	0.17	

Intersection Summary

Area Type:	Other
Cycle Length:	110
Actuated Cycle Length:	110
Offset:	18 (16%), Referenced to phase 4:EBTL and 8:WBTL, Start of Green
Natural Cycle:	105
Control Type:	Actuated-Coordinated
Maximum v/c Ratio:	1.21
Intersection Signal Delay:	57.5
Intersection LOS:	E
Intersection Capacity Utilization:	95.3%
ICU Level of Service:	F
Analysis Period (min):	15
~	Volume exceeds capacity, queue is theoretically infinite. Queue shown is maximum after two cycles.
#	95th percentile volume exceeds capacity, queue may be longer. Queue shown is maximum after two cycles.
m	Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 204: Jefferson St & Chicago Ave



Bally's Chicago Casino
305: Canal St/Jefferson St & Grand Ave

2032 Future with PD1426
Timing Plan: Friday Casino PH



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	238	513	5	10	449	572	19	0	10	179	0	233
Future Volume (vph)	238	513	5	10	449	572	19	0	10	179	0	233
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	100		0	0		0	0		0	0		0
Storage Lanes	1		0	0		0	0		0	0		1
Taper Length (ft)	100			25			25			135		
Lane Util. Factor	1.00	0.95	0.95	0.95	0.95	0.95	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor		1.00			0.92			0.99			0.98	
Frt		0.999			0.917			0.954				0.850
Flt Protected	0.950							0.968			0.950	
Satd. Flow (prot)	1770	3534	0	0	2970	0	0	1698	0	0	1770	1583
Flt Permitted	0.117				0.948			0.968			0.950	
Satd. Flow (perm)	218	3534	0	0	2815	0	0	1698	0	0	1726	1583
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		1			328			109				253
Link Speed (mph)		30			30			30				30
Link Distance (ft)		375			838			269				430
Travel Time (s)		8.5			19.0			6.1				9.8
Confl. Peds. (#/hr)	56		17	17		56			18	18		
Confl. Bikes (#/hr)						2						
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	259	558	5	11	488	622	21	0	11	195	0	253
Shared Lane Traffic (%)												
Lane Group Flow (vph)	259	563	0	0	1121	0	0	32	0	0	195	253
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		12			0			0				0
Link Offset(ft)		0			0			0				0
Crosswalk Width(ft)		16			16			16				16
Two way Left Turn Lane												
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15		9	15		9	15		9	15		9
Number of Detectors	1	2		1	2		1	2		1	2	1
Detector Template	Left	Thru		Left	Thru		Left	Thru		Left	Thru	Right
Leading Detector (ft)	20	100		20	100		20	100		20	100	20
Trailing Detector (ft)	0	0		0	0		0	0		0	0	0
Detector 1 Position(ft)	0	0		0	0		0	0		0	0	0
Detector 1 Size(ft)	20	6		20	6		20	6		20	6	20
Detector 1 Type	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	Cl+Ex
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	0.0
Detector 2 Position(ft)		94			94			94				94
Detector 2 Size(ft)		6			6			6				6
Detector 2 Type		Cl+Ex			Cl+Ex			Cl+Ex				Cl+Ex
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0				0.0

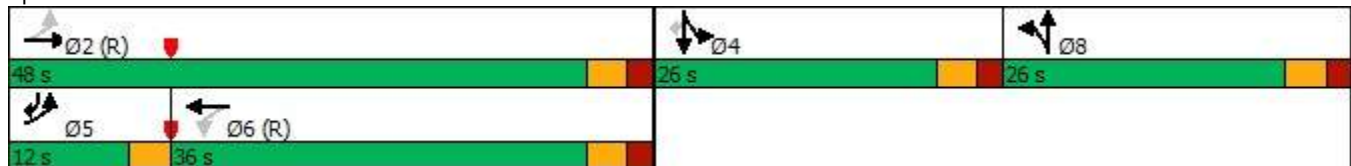


Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Turn Type	pm+pt	NA		Perm	NA		Split	NA		Split	NA	pm+ov
Protected Phases	5	2			6		8	8		4	4	5
Permitted Phases	2			6								4
Detector Phase	5	2		6	6		8	8		4	4	5
Switch Phase												
Minimum Initial (s)	5.0	5.0		5.0	5.0		5.0	5.0		5.0	5.0	5.0
Minimum Split (s)	10.0	26.0		26.0	26.0		26.0	26.0		23.0	23.0	10.0
Total Split (s)	12.0	48.0		36.0	36.0		26.0	26.0		26.0	26.0	12.0
Total Split (%)	12.0%	48.0%		36.0%	36.0%		26.0%	26.0%		26.0%	26.0%	12.0%
Maximum Green (s)	9.0	43.0		31.0	31.0		21.0	21.0		21.0	21.0	9.0
Yellow Time (s)	3.0	3.0		3.0	3.0		3.0	3.0		3.0	3.0	3.0
All-Red Time (s)	0.0	2.0		2.0	2.0		2.0	2.0		2.0	2.0	0.0
Lost Time Adjust (s)	0.0	0.0			0.0			0.0			0.0	0.0
Total Lost Time (s)	3.0	5.0			5.0			5.0			5.0	3.0
Lead/Lag	Lead			Lag	Lag							Lead
Lead-Lag Optimize?	Yes			Yes	Yes							Yes
Vehicle Extension (s)	3.0	3.0		3.0	3.0		3.0	3.0		3.0	3.0	3.0
Recall Mode	None	C-Max		C-Max	C-Max		None	None		None	None	None
Walk Time (s)		7.0		7.0	7.0		7.0	7.0				
Flash Dont Walk (s)		14.0		14.0	14.0		14.0	14.0				
Pedestrian Calls (#/hr)		0		0	0		0	0				
Act Effct Green (s)	69.7	67.7			46.0			5.5			16.0	37.9
Actuated g/C Ratio	0.70	0.68			0.46			0.06			0.16	0.38
v/c Ratio	0.59	0.24			0.76			0.16			0.69	0.33
Control Delay	19.3	7.7			21.7			1.8			52.0	2.7
Queue Delay	0.0	0.3			0.0			0.0			0.0	0.0
Total Delay	19.3	8.0			21.7			1.8			52.0	2.7
LOS	B	A			C			A			D	A
Approach Delay		11.5			21.7			1.8			24.2	
Approach LOS		B			C			A			C	
90th %ile Green (s)	21.9	58.5		33.6	33.6		5.5	5.5		21.0	21.0	21.9
90th %ile Term Code	Gap	Coord		Coord	Coord		Gap	Gap		Max	Max	Gap
70th %ile Green (s)	18.5	60.9		39.4	39.4		5.5	5.5		18.6	18.6	18.5
70th %ile Term Code	Gap	Coord		Coord	Coord		Gap	Gap		Gap	Gap	Gap
50th %ile Green (s)	17.4	63.3		42.9	42.9		5.5	5.5		16.2	16.2	17.4
50th %ile Term Code	Gap	Coord		Coord	Coord		Gap	Gap		Gap	Gap	Gap
30th %ile Green (s)	16.0	76.2		57.2	57.2		0.0	0.0		13.8	13.8	16.0
30th %ile Term Code	Gap	Coord		Coord	Coord		Skip	Skip		Gap	Gap	Gap
10th %ile Green (s)	19.7	79.7		57.0	57.0		0.0	0.0		10.3	10.3	19.7
10th %ile Term Code	Gap	Coord		Coord	Coord		Skip	Skip		Gap	Gap	Gap
Queue Length 50th (ft)	77	75			234			0			118	0
Queue Length 95th (ft)	165	116			#431			0			183	31
Internal Link Dist (ft)		295			758			189			350	
Turn Bay Length (ft)	100											
Base Capacity (vph)	442	2393			1472			442			371	756
Starvation Cap Reductn	0	1145			0			0			0	0
Spillback Cap Reductn	0	0			0			0			0	0
Storage Cap Reductn	0	0			0			0			0	0
Reduced v/c Ratio	0.59	0.45			0.76			0.07			0.53	0.33

Intersection Summary

Area Type:	Other
Cycle Length:	100
Actuated Cycle Length:	100
Offset:	0 (0%), Referenced to phase 2:EBTL and 6:WBTL, Start of Green
Natural Cycle:	85
Control Type:	Actuated-Coordinated
Maximum v/c Ratio:	0.76
Intersection Signal Delay:	18.4
Intersection LOS:	B
Intersection Capacity Utilization:	77.2%
ICU Level of Service:	D
Analysis Period (min):	15
# 95th percentile volume exceeds capacity, queue may be longer. Queue shown is maximum after two cycles.	

Splits and Phases: 305: Canal St/Jefferson St & Grand Ave



Bally's Chicago Casino
400: Jefferson St & Garage Entrance

2032 Future with PD1426
Timing Plan: Friday Casino PH



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕	↗	↖	↕	↗	↖	↕	↗
Traffic Volume (vph)	19	0	10	216	0	417	14	363	340	199	237	16
Future Volume (vph)	19	0	10	216	0	417	14	363	340	199	237	16
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0		0	0		0	60		100	205		0
Storage Lanes	0		0	0		1	1		0	0		0
Taper Length (ft)	25			25			55			70		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor		0.96			0.71	0.95	0.94		0.89	0.97	0.99	
Frt		0.954				0.850			0.850		0.991	
Flt Protected		0.968			0.950		0.950			0.950		
Satd. Flow (prot)	0	1706	0	0	1805	1615	1805	1863	646	1805	1478	0
Flt Permitted		0.687			0.755		0.590			0.381		
Satd. Flow (perm)	0	1190	0	0	1016	1538	1051	1863	575	701	1478	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		79				297			370			5
Link Speed (mph)		30			30			30				30
Link Distance (ft)		130			244			120				363
Travel Time (s)		3.0			5.5			2.7				8.3
Confl. Peds. (#/hr)	20		20	109		20	30		30	30		30
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	0%	0%	0%	0%	0%	0%	0%	2%	0%	0%	2%	0%
Parking (#/hr)									100		19	
Adj. Flow (vph)	21	0	11	235	0	453	15	395	370	216	258	17
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	32	0	0	235	453	15	395	370	216	275	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		0			0			12			12	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	2.95	1.00	1.31	1.00
Turning Speed (mph)	60		60	15		9	60		9	15		60
Number of Detectors	1	2		1	2	1	1	2	1	1	2	
Detector Template	Left	Thru		Left	Thru	Right	Left	Thru	Right	Left	Thru	
Leading Detector (ft)	20	100		20	100	20	20	100	20	20	100	
Trailing Detector (ft)	0	0		0	0	0	0	0	0	0	0	
Detector 1 Position(ft)	0	0		0	0	0	0	0	0	0	0	
Detector 1 Size(ft)	20	6		20	6	20	20	6	20	20	6	
Detector 1 Type	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Detector 1 Queue (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Detector 1 Delay (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Detector 2 Position(ft)		94			94			94			94	
Detector 2 Size(ft)		6			6			6			6	
Detector 2 Type		Cl+Ex			Cl+Ex			Cl+Ex			Cl+Ex	
Detector 2 Channel												

Bally's Chicago Casino
400: Jefferson St & Garage Entrance

2032 Future with PD1426
Timing Plan: Friday Casino PH



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Detector 2 Extend (s)		0.0			0.0			0.0			0.0	
Turn Type	Perm	NA		pm+pt	NA	custom	Perm	NA	pm+ov	pm+pt	NA	
Protected Phases		4		3	8	13		2	3	1	6	
Permitted Phases	4			8		8	2		2	6		
Detector Phase	4	4		3	8	13	2	2	3	1	6	
Switch Phase												
Minimum Initial (s)	1.0	1.0		5.0	5.0		5.0	5.0	5.0	5.0	5.0	
Minimum Split (s)	23.0	23.0		16.0	23.0		23.0	23.0	16.0	16.0	23.0	
Total Split (s)	23.0	23.0		16.0	39.0		55.0	55.0	16.0	16.0	71.0	
Total Split (%)	20.9%	20.9%		14.5%	35.5%		50.0%	50.0%	14.5%	14.5%	64.5%	
Maximum Green (s)	18.0	18.0		13.0	34.0		50.0	50.0	13.0	13.0	66.0	
Yellow Time (s)	3.0	3.0		3.0	3.0		3.0	3.0	3.0	3.0	3.0	
All-Red Time (s)	2.0	2.0		0.0	2.0		2.0	2.0	0.0	0.0	2.0	
Lost Time Adjust (s)		0.0			0.0		0.0	0.0	0.0	0.0	0.0	
Total Lost Time (s)		5.0			5.0		5.0	5.0	3.0	3.0	5.0	
Lead/Lag	Lag	Lag		Lead			Lag	Lag	Lead	Lead		
Lead-Lag Optimize?	Yes	Yes		Yes			Yes	Yes	Yes	Yes		
Vehicle Extension (s)	3.0	3.0		3.0	3.0		3.0	3.0	3.0	3.0	3.0	
Recall Mode	None	None		None	Max		C-Max	C-Max	None	None	C-Max	
Walk Time (s)	7.0	7.0			7.0		7.0	7.0			7.0	
Flash Dont Walk (s)	11.0	11.0			11.0		11.0	11.0			11.0	
Pedestrian Calls (#/hr)	0	0			0		0	0			0	
Act Effct Green (s)		13.0			34.0	48.2	52.0	52.0	76.2	68.0	66.0	
Actuated g/C Ratio		0.12			0.31	0.44	0.47	0.47	0.69	0.62	0.60	
v/c Ratio		0.15			0.51	0.52	0.03	0.45	0.70	0.40	0.31	
Control Delay		1.6			35.2	8.8	16.7	21.9	9.9	4.6	3.9	
Queue Delay		0.0			0.0	0.1	0.0	0.0	0.0	0.0	0.5	
Total Delay		1.6			35.2	8.9	16.7	21.9	9.9	4.6	4.4	
LOS		A			D	A	B	C	A	A	A	
Approach Delay		1.6			17.9			16.1			4.5	
Approach LOS		A			B			B			A	
90th %ile Green (s)	18.0	18.0		13.0	34.0		50.0	50.0	13.0	13.0	66.0	
90th %ile Term Code	Hold	Hold		Max	MaxR		Coord	Coord	Max	Max	Coord	
70th %ile Green (s)	18.0	18.0		13.0	34.0		50.0	50.0	13.0	13.0	66.0	
70th %ile Term Code	Hold	Hold		Max	MaxR		Coord	Coord	Max	Max	Coord	
50th %ile Green (s)	18.0	18.0		13.0	34.0		51.8	51.8	13.0	11.2	66.0	
50th %ile Term Code	Hold	Hold		Max	MaxR		Coord	Coord	Max	Gap	Coord	
30th %ile Green (s)	0.0	0.0		36.0	34.0		53.1	53.1	36.0	9.9	66.0	
30th %ile Term Code	Skip	Skip		Hold	MaxR		Coord	Coord	Hold	Gap	Coord	
10th %ile Green (s)	0.0	0.0		36.0	34.0		54.9	54.9	36.0	8.1	66.0	
10th %ile Term Code	Skip	Skip		Hold	MaxR		Coord	Coord	Hold	Gap	Coord	
Queue Length 50th (ft)		0			131	65	6	182	0	18	23	
Queue Length 95th (ft)		0			205	146	18	273	70	21	26	
Internal Link Dist (ft)		50			164			40			283	
Turn Bay Length (ft)							60		100	205		
Base Capacity (vph)		260			458	889	496	879	526	563	888	
Starvation Cap Reductn		0			0	0	0	0	0	0	294	
Spillback Cap Reductn		1			0	25	0	0	0	0	0	
Storage Cap Reductn		0			0	0	0	0	0	0	0	

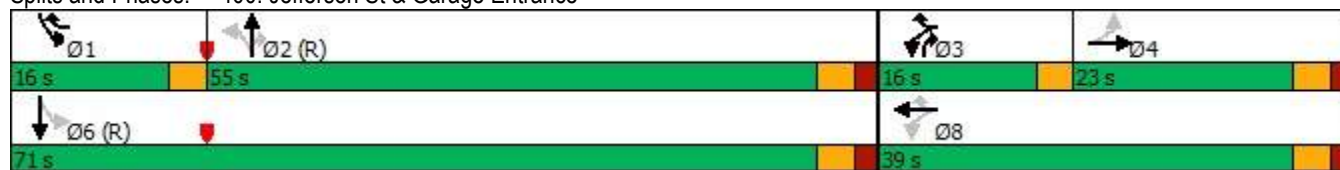


Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Reduced v/c Ratio		0.12			0.51	0.52	0.03	0.45	0.70	0.38	0.46	

Intersection Summary

Area Type:	Other
Cycle Length:	110
Actuated Cycle Length:	110
Offset:	23 (21%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green
Natural Cycle:	80
Control Type:	Actuated-Coordinated
Maximum v/c Ratio:	0.70
Intersection Signal Delay:	13.6
Intersection LOS:	B
Intersection Capacity Utilization	67.5%
ICU Level of Service	C
Analysis Period (min)	15

Splits and Phases: 400: Jefferson St & Garage Entrance



Intersection												
Int Delay, s/veh	1											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	SEL	SET	SER	NWL	NWT	NWR
Lane Configurations		↕					↕	↕	↕		↕	↕
Traffic Vol, veh/h	12	3	8	0	0	0	45	411	7	9	705	81
Future Vol, veh/h	12	3	8	0	0	0	45	411	7	9	705	81
Conflicting Peds, #/hr	0	0	0	0	0	0	30	0	0	0	0	54
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	0	100	-	-	50	-	50
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	92	92	92	92	92	92	92	92	92	92	92	92
Heavy Vehicles, %	0	0	0	2	2	2	0	2	2	2	2	0
Mvmt Flow	13	3	9	0	0	0	49	447	8	10	766	88

Major/Minor	Minor1		Minor2		Major1		Major2					
Conflicting Flow All	1379	1477	451	-	-	820	908	0	0	455	0	0
Stage 1	549	549	-	-	-	-	-	-	-	-	-	-
Stage 2	830	928	-	-	-	-	-	-	-	-	-	-
Critical Hdwy	7.1	6.5	6.2	-	-	6.22	4.1	-	-	4.12	-	-
Critical Hdwy Stg 1	6.1	5.5	-	-	-	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.1	5.5	-	-	-	-	-	-	-	-	-	-
Follow-up Hdwy	3.5	4	3.3	-	-	3.318	2.2	-	-	2.218	-	-
Pot Cap-1 Maneuver	123	127	613	0	0	375	758	-	-	1106	-	-
Stage 1	524	520	-	0	0	-	-	-	-	-	-	-
Stage 2	367	349	-	0	0	-	-	-	-	-	-	-
Platoon blocked, %								-	-	-	-	-
Mov Cap-1 Maneuver	116	111	613	-	-	356	719	-	-	1106	-	-
Mov Cap-2 Maneuver	116	111	-	-	-	-	-	-	-	-	-	-
Stage 1	488	485	-	-	-	-	-	-	-	-	-	-
Stage 2	364	328	-	-	-	-	-	-	-	-	-	-

Approach	EB	WB	SE	NW
HCM Control Delay, s	31.6	0	1	0.1
HCM LOS	D	A		

Minor Lane/Major Mvmt	NWL	NWT	NWR	EBLn1WBLn1	SEL	SET	SER
Capacity (veh/h)	1106	-	-	160	-	719	-
HCM Lane V/C Ratio	0.009	-	-	0.156	-	0.068	-
HCM Control Delay (s)	8.3	-	-	31.6	0	10.4	-
HCM Lane LOS	A	-	-	D	A	B	-
HCM 95th %tile Q(veh)	0	-	-	0.5	-	0.2	-

Intersection												
Int Delay, s/veh	3.2											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	SEL	SET	SER	NWL	NWT	NWR
Lane Configurations		↕				↕	↕	↕		↕	↕	
Traffic Vol, veh/h	12	0	11	0	0	116	51	365	2	16	669	91
Future Vol, veh/h	12	0	11	0	0	116	51	365	2	16	669	91
Conflicting Peds, #/hr	20	0	20	0	0	30	30	0	20	20	0	27
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	0	100	-	-	50	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	92	92	92	92	92	92	92	92	92	92	92	92
Heavy Vehicles, %	2	0	2	0	0	0	0	2	2	2	2	0
Mvmt Flow	13	0	12	0	0	126	55	397	2	17	727	99

Major/Minor	Minor1		Minor2		Major1		Major2					
Conflicting Flow All	1432	1418	438	-	-	837	856	0	0	419	0	0
Stage 1	528	528	-	-	-	-	-	-	-	-	-	-
Stage 2	904	890	-	-	-	-	-	-	-	-	-	-
Critical Hdwy	7.12	6.5	6.22	-	-	6.2	4.1	-	-	4.12	-	-
Critical Hdwy Stg 1	6.12	5.5	-	-	-	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.12	5.5	-	-	-	-	-	-	-	-	-	-
Follow-up Hdwy	3.518	4	3.318	-	-	3.3	2.2	-	-	2.218	-	-
Pot Cap-1 Maneuver	112	138	619	0	0	370	793	-	-	1140	-	-
Stage 1	534	531	-	0	0	-	-	-	-	-	-	-
Stage 2	331	364	-	0	0	-	-	-	-	-	-	-
Platoon blocked, %								-	-	-	-	-
Mov Cap-1 Maneuver	63	119	596	-	-	346	763	-	-	1118	-	-
Mov Cap-2 Maneuver	63	119	-	-	-	-	-	-	-	-	-	-
Stage 1	486	483	-	-	-	-	-	-	-	-	-	-
Stage 2	201	345	-	-	-	-	-	-	-	-	-	-

Approach	EB		WB		SE		NW	
HCM Control Delay, s	47.1		21.3		1.2		0.2	
HCM LOS	E		C					

Minor Lane/Major Mvmt	NWL	NWT	NWR	EBLn1	WBLn1	SEL	SET	SER
Capacity (veh/h)	1118	-	-	110	346	763	-	-
HCM Lane V/C Ratio	0.016	-	-	0.227	0.364	0.073	-	-
HCM Control Delay (s)	8.3	-	-	47.1	21.3	10.1	-	-
HCM Lane LOS	A	-	-	E	C	B	-	-
HCM 95th %tile Q(veh)	0	-	-	0.8	1.6	0.2	-	-

Intersection												
Int Delay, s/veh	2.1											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	SEL	SET	SER	NWL	NWT	NWR
Lane Configurations		↕			↕			↕			↕	↕
Traffic Vol, veh/h	12	24	0	27	0	11	2	380	23	1	785	20
Future Vol, veh/h	12	24	0	27	0	11	2	380	23	1	785	20
Conflicting Peds, #/hr	0	0	0	20	0	20	20	0	0	0	0	20
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	100
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	92	92	92	92	92	92	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	13	26	0	29	0	12	2	413	25	1	853	22

























Major/Minor	Minor1		Minor2		Major1		Major2					
Conflicting Flow All	1322	1327	239	1119	1317	893	895	0	0	438	0	0
Stage 1	430	430	-	875	875	-	-	-	-	-	-	-
Stage 2	892	897	-	244	442	-	-	-	-	-	-	-
Critical Hdwy	7.33	6.53	6.93	7.33	6.53	6.23	4.13	-	-	4.13	-	-
Critical Hdwy Stg 1	6.53	5.53	-	6.13	5.53	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.13	5.53	-	6.53	5.53	-	-	-	-	-	-	-
Follow-up Hdwy	3.519	4.019	3.319	3.519	4.019	3.319	2.219	-	-	2.219	-	-
Pot Cap-1 Maneuver	124	155	763	172	157	339	756	-	-	1120	-	-
Stage 1	574	583	-	343	366	-	-	-	-	-	-	-
Stage 2	336	357	-	739	576	-	-	-	-	-	-	-
Platoon blocked, %								-	-	-	-	-
Mov Cap-1 Maneuver	117	151	748	143	153	326	742	-	-	1120	-	-
Mov Cap-2 Maneuver	117	151	-	143	153	-	-	-	-	-	-	-
Stage 1	572	581	-	335	358	-	-	-	-	-	-	-
Stage 2	317	350	-	690	574	-	-	-	-	-	-	-

Approach	EB		WB		SE		NW	
HCM Control Delay, s	41.1		32.6		0		0	
HCM LOS	E		D					

Minor Lane/Major Mvmt	NWL	NWT	NWR	EBLn1	WBLn1	SEL	SET	SER
Capacity (veh/h)	1120	-	-	138	171	742	-	-
HCM Lane V/C Ratio	0.001	-	-	0.284	0.242	0.003	-	-
HCM Control Delay (s)	8.2	0	-	41.1	32.6	9.9	0	-
HCM Lane LOS	A	A	-	E	D	A	A	-
HCM 95th %tile Q(veh)	0	-	-	1.1	0.9	0	-	-

Bally's Chicago Casino
204: Jefferson St & Chicago Ave

2032 Future with PD1426
Timing Plan: Saturday Casino PH

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	17	860	318	240	617	12	300	2	650	18	11	35
Future Volume (vph)	17	860	318	240	617	12	300	2	650	18	11	35
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	80		55	95		55	190		190	0		0
Storage Lanes	1		1	1		1	0		1	1		0
Taper Length (ft)	100			90			75			25		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor	0.97		0.91			0.85	0.87		0.91	0.92	0.87	
Frt			0.850			0.850			0.850		0.886	
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1770	1863	1583	1770	1863	1583	1770	1863	1583	1770	1438	0
Flt Permitted	0.249			0.092			0.618			0.757		
Satd. Flow (perm)	450	1863	1436	171	1863	1338	1001	1863	1436	1294	1438	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)			139			109			431			38
Link Speed (mph)		30			30			20				30
Link Distance (ft)		390			893			363				230
Travel Time (s)		8.9			20.3			12.4				5.2
Confl. Peds. (#/hr)	50		50	50		50	50		50	50		50
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	18	935	346	261	671	13	326	2	707	20	12	38
Shared Lane Traffic (%)												
Lane Group Flow (vph)	18	935	346	261	671	13	326	2	707	20	50	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		12			12			12				12
Link Offset(ft)		0			0			0				0
Crosswalk Width(ft)		16			16			16				16
Two way Left Turn Lane												
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Number of Detectors	1	2	1	1	2	1	1	2	1	1	2	
Detector Template	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	
Leading Detector (ft)	20	100	20	20	100	20	20	100	20	20	100	
Trailing Detector (ft)	0	0	0	0	0	0	0	0	0	0	0	
Detector 1 Position(ft)	0	0	0	0	0	0	0	0	0	0	0	
Detector 1 Size(ft)	20	6	20	20	6	20	20	6	20	20	6	
Detector 1 Type	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Detector 2 Position(ft)		94			94			94				94
Detector 2 Size(ft)		6			6			6				6
Detector 2 Type		Cl+Ex			Cl+Ex			Cl+Ex				Cl+Ex
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0				0.0
Turn Type	pm+pt	NA	Perm	pm+pt	NA	Perm	pm+pt	NA	Perm	pm+pt	NA	
Protected Phases	7	4		3	8		5	2		1		6

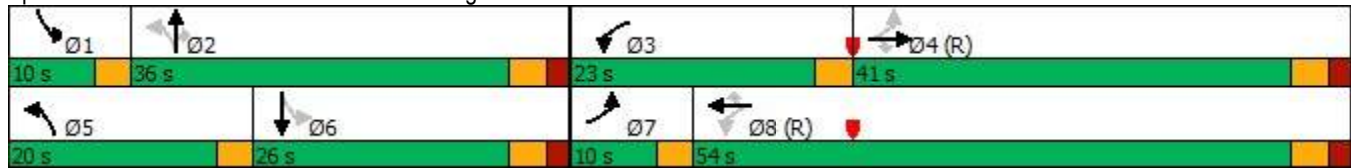


Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Permitted Phases	4		4	8		8	2		2	6		
Detector Phase	7	4	4	3	8	8	5	2	2	1	6	
Switch Phase												
Minimum Initial (s)	3.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	3.0	5.0	
Minimum Split (s)	8.0	38.0	38.0	16.0	50.0	50.0	16.0	36.0	36.0	8.0	25.0	
Total Split (s)	10.0	41.0	41.0	23.0	54.0	54.0	20.0	36.0	36.0	10.0	26.0	
Total Split (%)	9.1%	37.3%	37.3%	20.9%	49.1%	49.1%	18.2%	32.7%	32.7%	9.1%	23.6%	
Maximum Green (s)	7.0	36.0	36.0	20.0	49.0	49.0	17.0	31.0	31.0	7.0	21.0	
Yellow Time (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	
All-Red Time (s)	0.0	2.0	2.0	0.0	2.0	2.0	0.0	2.0	2.0	0.0	2.0	
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Lost Time (s)	3.0	5.0	5.0	3.0	5.0	5.0	3.0	5.0	5.0	3.0	5.0	
Lead/Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	
Recall Mode	None	C-Max	C-Max	None	C-Max	C-Max	None	None	None	None	None	
Walk Time (s)		4.0	4.0		4.0	4.0		4.0	4.0		7.0	
Flash Dont Walk (s)		16.0	16.0		16.0	16.0		16.0	16.0		11.0	
Pedestrian Calls (#/hr)		0	0		0	0		0	0		0	
Act Effct Green (s)	50.1	42.1	42.1	62.5	56.7	56.7	41.5	35.5	35.5	24.6	17.9	
Actuated g/C Ratio	0.46	0.38	0.38	0.57	0.52	0.52	0.38	0.32	0.32	0.22	0.16	
v/c Ratio	0.06	1.31	0.55	0.81	0.70	0.02	0.62	0.00	0.94	0.06	0.19	
Control Delay	12.4	173.5	16.9	44.0	26.8	0.1	26.5	27.5	30.5	22.1	17.7	
Queue Delay	0.0	0.7	0.6	0.0	53.0	0.0	2.8	0.0	9.7	0.0	0.0	
Total Delay	12.4	174.2	17.5	44.0	79.8	0.1	29.3	27.5	40.1	22.1	17.7	
LOS	B	F	B	D	E	A	C	C	D	C	B	
Approach Delay		130.2			68.9			36.7			19.0	
Approach LOS		F			E			D			B	
90th %ile Green (s)	6.8	36.0	36.0	20.0	49.2	49.2	17.0	31.0	31.0	7.0	21.0	
90th %ile Term Code	Gap	Coord	Coord	Max	Coord	Coord	Max	Max	Max	Max	Hold	
70th %ile Green (s)	6.3	36.6	36.6	19.4	49.7	49.7	17.0	31.3	31.3	6.7	21.0	
70th %ile Term Code	Gap	Coord	Coord	Gap	Coord	Coord	Max	Max	Max	Gap	Hold	
50th %ile Green (s)	0.0	39.9	39.9	16.1	59.0	59.0	17.0	41.0	41.0	0.0	21.0	
50th %ile Term Code	Skip	Coord	Coord	Gap	Coord	Coord	Max	Max	Max	Skip	Hold	
30th %ile Green (s)	0.0	43.2	43.2	12.8	59.0	59.0	17.0	41.0	41.0	0.0	21.0	
30th %ile Term Code	Skip	Coord	Coord	Gap	Coord	Coord	Max	Max	Max	Skip	Hold	
10th %ile Green (s)	0.0	54.6	54.6	9.1	66.7	66.7	35.3	33.3	33.3	0.0	0.0	
10th %ile Term Code	Skip	Coord	Coord	Gap	Coord	Coord	Hold	Gap	Gap	Skip	Skip	
Queue Length 50th (ft)	6	~874	86	124	328	0	167	1	199	9	7	
Queue Length 95th (ft)	m6	m#974	m96	213	563	0	265	m1	#234	25	41	
Internal Link Dist (ft)		310			813			283			150	
Turn Bay Length (ft)	80		55	95		55	190		190			
Base Capacity (vph)	293	712	634	387	960	742	522	601	755	329	305	
Starvation Cap Reductn	0	71	85	0	0	0	106	0	46	0	0	
Spillback Cap Reductn	0	0	0	0	431	0	0	0	0	0	8	
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	
Reduced v/c Ratio	0.06	1.46	0.63	0.67	1.27	0.02	0.78	0.00	1.00	0.06	0.17	

Intersection Summary

Area Type: Other
 Cycle Length: 110
 Actuated Cycle Length: 110
 Offset: 21 (19%), Referenced to phase 4:EBTL and 8:WBTL, Start of Green
 Natural Cycle: 105
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 1.31
 Intersection Signal Delay: 81.7 Intersection LOS: F
 Intersection Capacity Utilization 104.5% ICU Level of Service G
 Analysis Period (min) 15
 ~ Volume exceeds capacity, queue is theoretically infinite.
 Queue shown is maximum after two cycles.
 # 95th percentile volume exceeds capacity, queue may be longer.
 Queue shown is maximum after two cycles.
 m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 204: Jefferson St & Chicago Ave



Bally's Chicago Casino
305: Canal St/Jefferson St & Grand Ave

2032 Future with PD1426
Timing Plan: Saturday Casino PH



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	297	485	5	11	399	722	20	0	10	212	0	273
Future Volume (vph)	297	485	5	11	399	722	20	0	10	212	0	273
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	100		0	0		0	0		0	0		0
Storage Lanes	1		0	0		0	0		0	0		1
Taper Length (ft)	100			25			25			135		
Lane Util. Factor	1.00	0.95	0.95	0.95	0.95	0.95	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor		1.00			0.90			0.99			0.98	
Frt		0.999			0.904			0.955				0.850
Flt Protected	0.950							0.968			0.950	
Satd. Flow (prot)	1770	3534	0	0	2867	0	0	1705	0	0	1770	1583
Flt Permitted	0.106				0.948			0.968			0.950	
Satd. Flow (perm)	197	3534	0	0	2717	0	0	1705	0	0	1738	1583
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		1			429			109				297
Link Speed (mph)		30			30			30				30
Link Distance (ft)		375			838			269				430
Travel Time (s)		8.5			19.0			6.1				9.8
Confl. Peds. (#/hr)	56		10	10		56			13	13		
Confl. Bikes (#/hr)			6			10						
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	323	527	5	12	434	785	22	0	11	230	0	297
Shared Lane Traffic (%)												
Lane Group Flow (vph)	323	532	0	0	1231	0	0	33	0	0	230	297
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		12			0			0				0
Link Offset(ft)		0			0			0				0
Crosswalk Width(ft)		16			16			16				16
Two way Left Turn Lane												
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Number of Detectors	1	2		1	2		1	2		1	2	1
Detector Template	Left	Thru		Left	Thru		Left	Thru		Left	Thru	Right
Leading Detector (ft)	20	100		20	100		20	100		20	100	20
Trailing Detector (ft)	0	0		0	0		0	0		0	0	0
Detector 1 Position(ft)	0	0		0	0		0	0		0	0	0
Detector 1 Size(ft)	20	6		20	6		20	6		20	6	20
Detector 1 Type	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	Cl+Ex
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	0.0
Detector 2 Position(ft)		94			94			94				94
Detector 2 Size(ft)		6			6			6				6
Detector 2 Type		Cl+Ex			Cl+Ex			Cl+Ex				Cl+Ex
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0				0.0
Turn Type	pm+pt	NA		Perm	NA		Split	NA		Split	NA	pm+ov

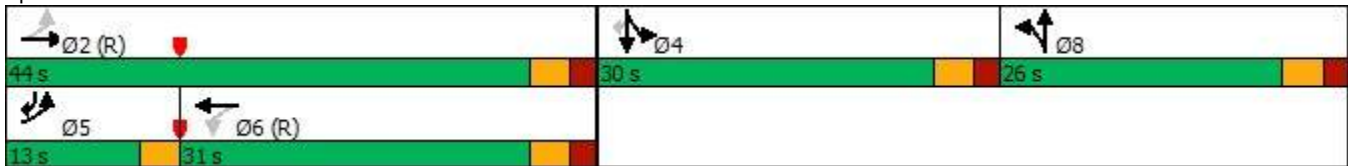


Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Protected Phases	5	2			6		8	8		4	4	5
Permitted Phases	2			6								4
Detector Phase	5	2		6	6		8	8		4	4	5
Switch Phase												
Minimum Initial (s)	5.0	5.0		5.0	5.0		5.0	5.0		5.0	5.0	5.0
Minimum Split (s)	10.0	30.0		30.0	30.0		26.0	26.0		26.0	26.0	10.0
Total Split (s)	13.0	44.0		31.0	31.0		26.0	26.0		30.0	30.0	13.0
Total Split (%)	13.0%	44.0%		31.0%	31.0%		26.0%	26.0%		30.0%	30.0%	13.0%
Maximum Green (s)	10.0	39.0		26.0	26.0		21.0	21.0		25.0	25.0	10.0
Yellow Time (s)	3.0	3.0		3.0	3.0		3.0	3.0		3.0	3.0	3.0
All-Red Time (s)	0.0	2.0		2.0	2.0		2.0	2.0		2.0	2.0	0.0
Lost Time Adjust (s)	0.0	0.0			0.0			0.0			0.0	0.0
Total Lost Time (s)	3.0	5.0			5.0			5.0			5.0	3.0
Lead/Lag	Lead			Lag	Lag							Lead
Lead-Lag Optimize?	Yes			Yes	Yes							Yes
Vehicle Extension (s)	3.0	3.0		3.0	3.0		3.0	3.0		3.0	3.0	3.0
Recall Mode	None	C-Max		C-Max	C-Max		None	None		None	None	None
Walk Time (s)		7.0		7.0	7.0		7.0	7.0				
Flash Dont Walk (s)		14.0		14.0	14.0		14.0	14.0				
Pedestrian Calls (#/hr)		0		0	0		0	0				
Act Effct Green (s)	67.5	65.5			36.6			5.5			18.2	47.3
Actuated g/C Ratio	0.68	0.66			0.37			0.06			0.18	0.47
v/c Ratio	0.60	0.23			0.97			0.17			0.72	0.33
Control Delay	23.3	8.7			41.7			1.9			50.6	2.0
Queue Delay	0.0	0.0			0.0			0.0			0.0	0.0
Total Delay	23.3	8.7			41.7			1.9			50.6	2.0
LOS	C	A			D			A			D	A
Approach Delay		14.2			41.7			1.9			23.2	
Approach LOS		B			D			A			C	
90th %ile Green (s)	26.0	55.0		26.0	26.0		5.5	5.5		24.5	24.5	26.0
90th %ile Term Code	Max	Coord		Coord	Coord		Gap	Gap		Gap	Gap	Max
70th %ile Green (s)	24.0	58.7		31.7	31.7		5.5	5.5		20.8	20.8	24.0
70th %ile Term Code	Gap	Coord		Coord	Coord		Gap	Gap		Gap	Gap	Gap
50th %ile Green (s)	23.0	61.3		35.3	35.3		5.5	5.5		18.2	18.2	23.0
50th %ile Term Code	Gap	Coord		Coord	Coord		Gap	Gap		Gap	Gap	Gap
30th %ile Green (s)	23.1	74.4		48.3	48.3		0.0	0.0		15.6	15.6	23.1
30th %ile Term Code	Gap	Coord		Coord	Coord		Skip	Skip		Gap	Gap	Gap
10th %ile Green (s)	33.4	78.3		41.9	41.9		0.0	0.0		11.7	11.7	33.4
10th %ile Term Code	Gap	Coord		Coord	Coord		Skip	Skip		Gap	Gap	Gap
Queue Length 50th (ft)	128	74			297			0			139	0
Queue Length 95th (ft)	228	120			#526			0			205	26
Internal Link Dist (ft)		295			758			189			350	
Turn Bay Length (ft)	100											
Base Capacity (vph)	540	2316			1267			444			442	904
Starvation Cap Reductn	0	0			0			0			0	0
Spillback Cap Reductn	0	0			0			0			0	0
Storage Cap Reductn	0	0			0			0			0	0
Reduced v/c Ratio	0.60	0.23			0.97			0.07			0.52	0.33

Intersection Summary

Area Type:	Other
Cycle Length:	100
Actuated Cycle Length:	100
Offset:	0 (0%), Referenced to phase 2:EBTL and 6:WBTL, Start of Green
Natural Cycle:	95
Control Type:	Actuated-Coordinated
Maximum v/c Ratio:	0.97
Intersection Signal Delay:	28.6
Intersection LOS:	C
Intersection Capacity Utilization:	83.4%
ICU Level of Service:	E
Analysis Period (min):	15
# 95th percentile volume exceeds capacity, queue may be longer.	
Queue shown is maximum after two cycles.	

Splits and Phases: 305: Canal St/Jefferson St & Grand Ave



Bally's Chicago Casino
400: Jefferson St & Garage Entrance

2032 Future with PD1426
Timing Plan: Saturday Casino PH



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕	↕	↕	↑	↕	↕	↕	↕
Traffic Volume (vph)	15	0	9	256	0	496	15	441	438	256	295	18
Future Volume (vph)	15	0	9	256	0	496	15	441	438	256	295	18
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0		0	0		0	60		100	205		0
Storage Lanes	0		0	0		1	1		0	0		0
Taper Length (ft)	25			25			55			70		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor		0.95			0.64	0.95	0.92		0.85		0.99	
Frt		0.948				0.850			0.850		0.991	
Flt Protected		0.970			0.950		0.950			0.950		
Satd. Flow (prot)	0	1693	0	0	1805	1615	1805	1863	646	1805	1430	0
Flt Permitted		0.698			0.755		0.556			0.250		
Satd. Flow (perm)	0	1200	0	0	911	1538	976	1863	551	475	1430	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		79				174			476			4
Link Speed (mph)		30			30			30				30
Link Distance (ft)		130			244			120				363
Travel Time (s)		3.0			5.5			2.7				8.3
Confl. Peds. (#/hr)	20		20	135		20	40		40	40		40
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	0%	0%	0%	0%	0%	0%	0%	2%	0%	0%	2%	0%
Parking (#/hr)									100		24	
Adj. Flow (vph)	16	0	10	278	0	539	16	479	476	278	321	20
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	26	0	0	278	539	16	479	476	278	341	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		0			0			12			12	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	2.95	1.00	1.37	1.00
Number of Detectors	1	2		1	2	1	1	2	1	1	2	
Detector Template	Left	Thru		Left	Thru	Right	Left	Thru	Right	Left	Thru	
Leading Detector (ft)	20	100		20	100	20	20	100	20	20	100	
Trailing Detector (ft)	0	0		0	0	0	0	0	0	0	0	
Detector 1 Position(ft)	0	0		0	0	0	0	0	0	0	0	
Detector 1 Size(ft)	20	6		20	6	20	20	6	20	20	6	
Detector 1 Type	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Detector 1 Queue (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Detector 1 Delay (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Detector 2 Position(ft)		94			94			94			94	
Detector 2 Size(ft)		6			6			6			6	
Detector 2 Type		Cl+Ex			Cl+Ex			Cl+Ex			Cl+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0			0.0	

Bally's Chicago Casino
400: Jefferson St & Garage Entrance

2032 Future with PD1426
Timing Plan: Saturday Casino PH

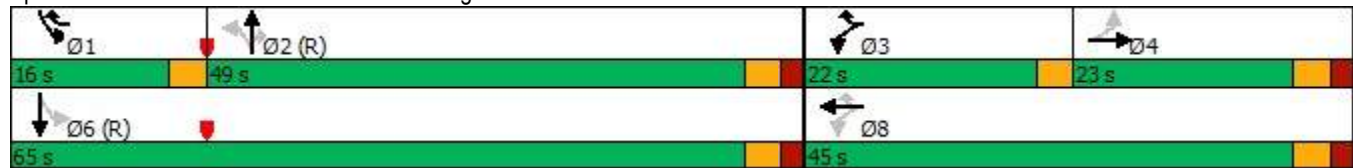


Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Turn Type	Perm	NA		pm+pt	NA	custom	Perm	NA	Perm	pm+pt	NA	
Protected Phases		4		3	8	13		2		1	6	
Permitted Phases	4			8		8	2		2	6		
Detector Phase	4	4		3	8	13	2	2	2	1	6	
Switch Phase												
Minimum Initial (s)	5.0	5.0		5.0	5.0		5.0	5.0	5.0	5.0	5.0	
Minimum Split (s)	23.0	23.0		16.0	23.0		22.0	22.0	22.0	13.0	23.0	
Total Split (s)	23.0	23.0		22.0	45.0		49.0	49.0	49.0	16.0	65.0	
Total Split (%)	20.9%	20.9%		20.0%	40.9%		44.5%	44.5%	44.5%	14.5%	59.1%	
Maximum Green (s)	18.0	18.0		19.0	40.0		44.0	44.0	44.0	13.0	60.0	
Yellow Time (s)	3.0	3.0		3.0	3.0		3.0	3.0	3.0	3.0	3.0	
All-Red Time (s)	2.0	2.0		0.0	2.0		2.0	2.0	2.0	0.0	2.0	
Lost Time Adjust (s)		0.0			0.0		0.0	0.0	0.0	0.0	0.0	
Total Lost Time (s)		5.0			5.0		5.0	5.0	5.0	3.0	5.0	
Lead/Lag	Lag	Lag		Lead			Lag	Lag	Lag	Lead		
Lead-Lag Optimize?	Yes	Yes		Yes			Yes	Yes	Yes	Yes		
Vehicle Extension (s)	3.0	3.0		3.0	3.0		3.0	3.0	3.0	3.0	3.0	
Recall Mode	None	None		Max	Max		C-Max	C-Max	C-Max	None	C-Max	
Walk Time (s)	7.0	7.0		7.0	7.0		7.0	7.0	7.0		7.0	
Flash Dont Walk (s)	11.0	11.0		11.0	11.0		10.0	10.0	10.0		11.0	
Pedestrian Calls (#/hr)	0	0		25	25		25	25	25		25	
Act Effct Green (s)		13.0		40.0	55.5	44.7	44.7	44.7	44.7	62.0	60.0	
Actuated g/C Ratio		0.12		0.36	0.50	0.41	0.41	0.41	0.41	0.56	0.55	
v/c Ratio		0.12		0.51	0.60	0.04	0.63	0.94	0.67	0.44		
Control Delay		1.2		30.3	14.9	20.7	30.9	32.6	13.4	6.2		
Queue Delay		0.0		0.0	0.0	0.0	0.1	0.0	0.8	0.8		
Total Delay		1.2		30.3	14.9	20.7	31.0	32.6	14.2	7.0		
LOS		A		C	B	C	C	C	B	A		
Approach Delay		1.2		20.2			31.6			10.2		
Approach LOS		A		C			C			B		
90th %ile Green (s)	18.0	18.0		19.0	40.0		44.0	44.0	44.0	13.0	60.0	
90th %ile Term Code	Hold	Hold		MaxR	MaxR		Coord	Coord	Coord	Max	Coord	
70th %ile Green (s)	18.0	18.0		19.0	40.0		44.0	44.0	44.0	13.0	60.0	
70th %ile Term Code	Hold	Hold		MaxR	MaxR		Coord	Coord	Coord	Max	Coord	
50th %ile Green (s)	18.0	18.0		19.0	40.0		44.0	44.0	44.0	13.0	60.0	
50th %ile Term Code	Hold	Hold		MaxR	MaxR		Coord	Coord	Coord	Max	Coord	
30th %ile Green (s)	0.0	0.0		42.0	40.0		44.6	44.6	44.6	12.4	60.0	
30th %ile Term Code	Skip	Skip		Hold	MaxR		Coord	Coord	Coord	Gap	Coord	
10th %ile Green (s)	0.0	0.0		42.0	40.0		47.0	47.0	47.0	10.0	60.0	
10th %ile Term Code	Skip	Skip		Hold	MaxR		Coord	Coord	Coord	Gap	Coord	
Queue Length 50th (ft)		0		146	164	7	269	0	25	64		
Queue Length 95th (ft)		0		223	268	22	385	#162	58	62		
Internal Link Dist (ft)		50		164			40			283		
Turn Bay Length (ft)							60		100	205		
Base Capacity (vph)		262		544	900	396	757	506	424	781		
Starvation Cap Reductn		0		0	0	0	0	0	28	199		
Spillback Cap Reductn		0		0	1	0	12	0	0	0		
Storage Cap Reductn		0		0	0	0	0	0	0	0		
Reduced v/c Ratio		0.10		0.51	0.60	0.04	0.64	0.94	0.70	0.59		

Intersection Summary

Area Type:	Other
Cycle Length:	110
Actuated Cycle Length:	110
Offset:	27 (25%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green
Natural Cycle:	90
Control Type:	Actuated-Coordinated
Maximum v/c Ratio:	0.94
Intersection Signal Delay:	22.0
Intersection LOS:	C
Intersection Capacity Utilization	77.0%
ICU Level of Service	D
Analysis Period (min)	15
# 95th percentile volume exceeds capacity, queue may be longer. Queue shown is maximum after two cycles.	

Splits and Phases: 400: Jefferson St & Garage Entrance



Intersection												
Int Delay, s/veh	1.4											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	SEL	SET	SER	NWL	NWT	NWR
Lane Configurations		↔				↔	↔	↔		↔	↔	↔
Traffic Vol, veh/h	14	3	8	0	0	0	58	495	7	9	880	105
Future Vol, veh/h	14	3	8	0	0	0	58	495	7	9	880	105
Conflicting Peds, #/hr	0	0	0	0	0	0	40	0	0	0	0	68
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	0	100	-	-	50	-	50
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	92	92	92	92	92	92	92	92	92	92	92	92
Heavy Vehicles, %	0	0	0	2	2	2	0	2	2	2	2	0
Mvmt Flow	15	3	9	0	0	0	63	538	8	10	957	114

Major/Minor	Minor1		Minor2		Major1		Major2					
Conflicting Flow All	1702	1827	542	-	-	1025	1139	0	0	546	0	0
Stage 1	668	668	-	-	-	-	-	-	-	-	-	-
Stage 2	1034	1159	-	-	-	-	-	-	-	-	-	-
Critical Hdwy	7.1	6.5	6.2	-	-	6.22	4.1	-	-	4.12	-	-
Critical Hdwy Stg 1	6.1	5.5	-	-	-	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.1	5.5	-	-	-	-	-	-	-	-	-	-
Follow-up Hdwy	3.5	4	3.3	-	-	3.318	2.2	-	-	2.218	-	-
Pot Cap-1 Maneuver	73	78	544	0	0	285	621	-	-	1023	-	-
Stage 1	451	459	-	0	0	-	-	-	-	-	-	-
Stage 2	283	272	-	0	0	-	-	-	-	-	-	-
Platoon blocked, %								-	-	-	-	-
Mov Cap-1 Maneuver	66	64	544	-	-	267	581	-	-	1023	-	-
Mov Cap-2 Maneuver	66	64	-	-	-	-	-	-	-	-	-	-
Stage 1	402	409	-	-	-	-	-	-	-	-	-	-
Stage 2	280	252	-	-	-	-	-	-	-	-	-	-

Approach	EB	WB	SE	NW
HCM Control Delay, s	60.6	0	1.2	0.1
HCM LOS	F	A		

Minor Lane/Major Mvmt	NWL	NWT	NWR	EBLn1WBLn1	SEL	SET	SER
Capacity (veh/h)	1023	-	-	91	-	581	-
HCM Lane V/C Ratio	0.01	-	-	0.299	-	0.109	-
HCM Control Delay (s)	8.6	-	-	60.6	0	11.9	-
HCM Lane LOS	A	-	-	F	A	B	-
HCM 95th %tile Q(veh)	0	-	-	1.1	-	0.4	-

Intersection												
Int Delay, s/veh	7.3											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	SEL	SET	SER	NWL	NWT	NWR
Lane Configurations		↕					↗	↖	↘		↙	↕
Traffic Vol, veh/h	14	0	12	0	0	139	64	433	3	17	847	116
Future Vol, veh/h	14	0	12	0	0	139	64	433	3	17	847	116
Conflicting Peds, #/hr	20	0	20	0	0	40	40	0	20	20	0	34
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	0	100	-	-	50	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	92	92	92	92	92	92	92	92	92	92	92	92
Heavy Vehicles, %	2	0	2	0	0	0	0	2	2	2	2	0
Mvmt Flow	15	0	13	0	0	151	70	471	3	18	921	126

Major/Minor	Minor1		Minor2		Major1		Major2					
Conflicting Flow All	1769	1756	513	-	-	1064	1087	0	0	494	0	0
Stage 1	633	633	-	-	-	-	-	-	-	-	-	-
Stage 2	1136	1123	-	-	-	-	-	-	-	-	-	-
Critical Hdwy	7.12	6.5	6.22	-	-	6.2	4.1	-	-	4.12	-	-
Critical Hdwy Stg 1	6.12	5.5	-	-	-	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.12	5.5	-	-	-	-	-	-	-	-	-	-
Follow-up Hdwy	3.518	4	3.318	-	-	3.3	2.2	-	-	2.218	-	-
Pot Cap-1 Maneuver	65	86	561	0	0	273	649	-	-	1070	-	-
Stage 1	468	476	-	0	0	-	-	-	-	-	-	-
Stage 2	246	283	-	0	0	-	-	-	-	-	-	-
Platoon blocked, %												
Mov Cap-1 Maneuver	22	70	540	-	-	249	616	-	-	1050	-	-
Mov Cap-2 Maneuver	22	70	-	-	-	-	-	-	-	-	-	-
Stage 1	407	414	-	-	-	-	-	-	-	-	-	-
Stage 2	91	264	-	-	-	-	-	-	-	-	-	-

Approach	EB		WB		SE		NW	
HCM Control Delay, s	219.6		39.5		1.5		0.1	
HCM LOS	F		E					

Minor Lane/Major Mvmt	NWL	NWT	NWR	EBLn1	WBLn1	SEL	SET	SER
Capacity (veh/h)	1050	-	-	39	249	616	-	-
HCM Lane V/C Ratio	0.018	-	-	0.725	0.607	0.113	-	-
HCM Control Delay (s)	8.5	-	-	219.6	39.5	11.6	-	-
HCM Lane LOS	A	-	-	F	E	B	-	-
HCM 95th %tile Q(veh)	0.1	-	-	2.7	3.6	0.4	-	-

Intersection												
Int Delay, s/veh	4											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	SEL	SET	SER	NWL	NWT	NWR
Lane Configurations		↕			↕			↕			↕	↕
Traffic Vol, veh/h	15	26	0	28	0	12	3	450	27	1	993	21
Future Vol, veh/h	15	26	0	28	0	12	3	450	27	1	993	21
Conflicting Peds, #/hr	0	0	0	20	0	20	20	0	0	0	0	20
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	100
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	92	92	92	92	92	92	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	16	28	0	30	0	13	3	489	29	1	1079	23

Major/Minor	Minor1		Minor2		Major1		Major2					
Conflicting Flow All	1629	1634	279	1386	1625	1119	1122	0	0	518	0	0
Stage 1	510	510	-	1101	1101	-	-	-	-	-	-	-
Stage 2	1119	1124	-	285	524	-	-	-	-	-	-	-
Critical Hdwy	7.33	6.53	6.93	7.33	6.53	6.23	4.13	-	-	4.13	-	-
Critical Hdwy Stg 1	6.53	5.53	-	6.13	5.53	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.13	5.53	-	6.53	5.53	-	-	-	-	-	-	-
Follow-up Hdwy	3.519	4.019	3.319	3.519	4.019	3.319	2.219	-	-	2.219	-	-
Pot Cap-1 Maneuver	74	101	719	111	102	251	620	-	-	1046	-	-
Stage 1	515	537	-	256	287	-	-	-	-	-	-	-
Stage 2	250	280	-	699	529	-	-	-	-	-	-	-
Platoon blocked, %								-	-	-	-	-
Mov Cap-1 Maneuver	68	98	705	82	99	242	608	-	-	1046	-	-
Mov Cap-2 Maneuver	68	98	-	82	99	-	-	-	-	-	-	-
Stage 1	511	533	-	249	281	-	-	-	-	-	-	-
Stage 2	232	274	-	645	525	-	-	-	-	-	-	-

Approach	EB		WB		SE		NW	
HCM Control Delay, s	88.5		64.3		0.1		0	
HCM LOS	F		F					

Minor Lane/Major Mvmt	NWL	NWT	NWR	EBLn1	WBLn1	SEL	SET	SER
Capacity (veh/h)	1046	-	-	84	102	608	-	-
HCM Lane V/C Ratio	0.001	-	-	0.531	0.426	0.005	-	-
HCM Control Delay (s)	8.4	0	-	88.5	64.3	11	0	-
HCM Lane LOS	A	A	-	F	F	B	A	-
HCM 95th %tile Q(veh)	0	-	-	2.3	1.8	0	-	-



Traffic Signal Warrant Analysis Worksheet



Date	11/15/2024
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Location Information

Intersection	Jefferson St and Chicago Ave
City	Chicago
County	Cook
District	1

Street Information

Major Street (EB/WB)	Chicago Ave
Minor Street (NB/SB)	Jefferson St

Review Information

Counts Used	2026 Future with Entertainment District Traffic Volumes
Count Dates	-
Date Reviewed	11/15/2024
Reviewed By	MFM

Warrant Information

Speed Limit of Major Street	30
Number of Lanes on Major	1
Number of Lanes on Minor	1
Isolated Community?	No
SRA Route Number	N/A

Jefferson St and Chicago Ave

City: **Chicago**
 County: **Cook**
 District: **1**

Major Chicago Ave
Minor Jefferson St

State Of Illinois
 Department of Transportation
 Bureau of Traffic

Date: 11/15/2024

SUMMARY OF TRAFFIC SURVEY

Route:	TRAFFIC FROM EAST Chicago Ave E. of: Jefferson St Going				TRAFFIC FROM WEST Chicago Ave W. of: Jefferson St Going				TOTAL NORTH AND SOUTH	TRAFFIC FROM NORTH Jefferson St N. of: Chicago Ave Going				TRAFFIC FROM SOUTH Jefferson St S. of: Chicago Ave Going				TOTAL NORTH AND SOUTH	GRAND TOTAL
	SOUTH ↩	WEST ←	NORTH ↶	TOTAL	NORTH ↷	EAST →	SOUTH ↘	TOTAL		EAST ↙	SOUTH ↓	WEST ↵	TOTAL	WEST ↶	NORTH ↑	EAST ↷	TOTAL		
START TIME																			
AM	103	808	0	911	0	1,307	137	1,444	2,355	0	0	0	0	117	0	262	379	379	2,734
PM	136	1,329	0	1,465	0	1,087	181	1,268	2,733	0	0	0	0	182	0	411	593	593	3,326
SAT	227	616	0	843	0	860	303	1,163	2,006	0	0	0	0	281	0	631	912	912	2,918
55th % AM	57	444	0	501	0	719	75	794	1,295	0	0	0	0	64	0	147	211	211	1,506
55th % PM	75	731	0	806	0	598	100	698	1,504	0	0	0	0	100	0	230	330	330	1,834
55th % Sat	125	339	0	464	0	473	167	640	1,104	0	0	0	0	155	0	353	508	508	1,612

REVIEW INFORMATION

COUNTS USED: 2026 Future with Entertainment District Traffic Volumes
 COUNT DATE(S): -
 DATA REVIEWED: 11/15/2024
 REVIEWED BY: MFM

RIGHT TURN FACTORIZATION SHEET

INTERSECTION: Jefferson St and Chicago Ave
 MUNICIPALITY: Chicago

COUNTY: Cook

DIR	PEAK HOUR	MINOR STREET				CRITICAL MAINLINE APPROACH VOLUME PER LANE	BASE RIGHT TURN REDUCTION %	MAINLINE CONGESTION FACTOR %	ADJUSTED RIGHT TURN REDUCTION %	ADJUSTED RIGHT TURNS	ADJUSTED MINOR STREET VOLUME
		STREET NAME		Jefferson St							
		CONFIG. #	CRIT. MAINLINE LANE #	LEFT	THROUGH						
NB	AM	117	0	262	379	1,444	75%	0%	75%	66	183
NB	PM	182	0	411	593	1,268	75%	0%	75%	103	285
NB	SAT	281	0	631	912	1,163	75%	0%	75%	158	439
NB	55th AM	64	0	147	211	444	75%	5%	70%	44	101
NB	55th PM	100	0	230	330	731	75%	20%	55%	104	157
NB	55th Sat	155	0	353	508	339	75%	0%	75%	88	241

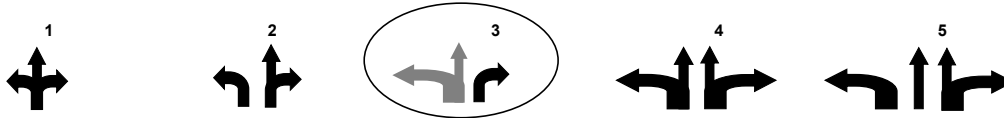
MAINLINE CONGESTION FACTORS	
VOLUMES	FACTOR %
0-399	0%
400-499	5%
500-599	10%
600-699	15%
700-799	20%
800-899	25%
900-999	30%
1000-1099	35%
1100-1199	40%
1200-1299	45%
1300-1399	50%
1400-1499	55%

REVIEW INFORMATION

COUNTS USED: 2026 Future with Entertainment District Traffic Volumes
 COUNT DATE(S): -
 DATE REVIEWEC 11/15/2024
 REVIEWED BY: MFM

RIGHT TURN FACTORIZATION SHEET (CONT.)

LANE CONFIGURATIONS



Any configuration with an exclusive right turn lane (usually up to 600 ft. long)

PH	LEFT	THROUGH	RIGHT	TOTAL (A)	0.7A	0.35A	3T	T/3	(T+L)	(T+R)	3R	3L	T/2	T/4	L=T=R (+/-)
AM	117	0	262	379	265	133	0	0	117	262	786	351	0	0	NO
PM	182	0	411	593	415	208	0	0	182	411	1233	546	0	0	NO
SAT	281	0	631	912	638	319	0	0	281	631	1893	843	0	0	NO
55th AM	64	0	147	211	148	74	0	0	64	147	441	192	0	0	NO
55th PM	100	0	230	330	231	116	0	0	100	230	690	300	0	0	NO
55th Sat	155	0	353	508	356	178	0	0	155	353	1059	465	0	0	NO

PH	BASE REDUCTION				
	CONFIG 1	CONFIG 2	CONFIG 3	CONFIG 4	CONFIG 5
AM	40%	60%	75%	65%	75%
PM	40%	60%	75%	65%	75%
SAT	40%	60%	75%	65%	75%
55th AM	40%	60%	75%	65%	75%
55th PM	40%	60%	75%	65%	75%
55th Sat	40%	60%	75%	65%	75%

SIGNAL WARRANT REVIEW SHEET

District #1

SRA: N/A
 YES NO

INTERSECTION: Jefferson St and Chicago Ave
 MUNICIPALITY: Chicago

COUNTY: Cook

Speed Limit of Major Route 30
 Number of Lanes of Major Approach 1

Isolated Community with Population < 10, No
 Number of Lanes of Minor Approach 1

HOUR BEGIN	MAJOR STREET VOLUME (both approaches)	ADJ. MINOR STREET VOLUME (higher volume approaches)	Check any hours which meet the following Warrants			
			WARRANT 1			
			A	B	WARRANT 1 A/B: 8 hrs of both	
				80% of A	80% of B	
AM	2,355	183	X	X	X	X
PM	2,733	285	X	X	X	X
SAT	2,006	439	X	X	X	X
55th % AM	1,295	101		X		X
55th % PM	1,504	157	X	X	X	X
55th % SAT	1,104	241	X	X	X	X

Volume Requirements:	Met:	Yes	Yes	Yes	Yes
	MAJOR:	500	750	400	600
	MINOR:	150	100	120	60

REVIEW INFORMATION

COUNTS USED: 2026 Future with Entertainment District Traffic Volumes
 COUNT DATE(S): -
 DATE REVIEWED: 11/15/2024
 REVIEWED BY: MFM

Comments

WARRANT 1	YES <input checked="" type="radio"/>	NO <input type="radio"/>	N/A <input type="radio"/>
Warrant 1 is met if any of the following Conditions are met:			
• CONDITION A Minum Vehicular Volume	YES <input checked="" type="radio"/>	NO <input type="radio"/>	N/A <input type="radio"/>
• CONDITION B Interruption of Continuous Traffic	YES <input checked="" type="radio"/>	NO <input type="radio"/>	N/A <input type="radio"/>
• CONDITION A/B Combination of Warrant 1s	YES <input checked="" type="radio"/>	NO <input type="radio"/>	N/A <input type="radio"/>
WARRANT 2 Four Hour Volume	YES <input type="radio"/>	NO <input type="radio"/>	N/A <input checked="" type="radio"/>
WARRANT 3 Peak Hour Volume	YES <input type="radio"/>	NO <input type="radio"/>	N/A <input checked="" type="radio"/>
WARRANT 4 Pedestrian Volume	YES <input type="radio"/>	NO <input type="radio"/>	N/A <input checked="" type="radio"/>
WARRANT 5 School Crossing	YES <input type="radio"/>	NO <input type="radio"/>	N/A <input checked="" type="radio"/>
WARRANT 6 Coordinated Signal System	YES <input type="radio"/>	NO <input type="radio"/>	N/A <input checked="" type="radio"/>
WARRANT 7 Accidents Experience	YES <input type="radio"/>	NO <input type="radio"/>	N/A <input checked="" type="radio"/>
WARRANT 8 Roadway Network	YES <input type="radio"/>	NO <input type="radio"/>	N/A <input checked="" type="radio"/>
WARRANT 9 Intersection Near a Grade Crossing	YES <input type="radio"/>	NO <input type="radio"/>	N/A <input checked="" type="radio"/>

Date	11/15/2024
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Location Information

Intersection	Grand Ave & Jefferson St
City	Chicago
County	Cook
District	1

Street Information

Major Street (EB/WB)	Grand Ave
Minor Street (NB/SB)	Jefferson St

Review Information

Counts Used	2026 Future w/Entertainment District Traffic Volumes
Count Dates	-
Date Reviewed	11/15/2024
Reviewed By	MFM

Warrant Information

Speed Limit of Major Street	30
Number of Lanes on Major	2
Number of Lanes on Minor	2
Isolated Community?	No
SRA Route Number	N/A

Grand Ave & Jefferson St

City: Chicago
 County: Cook
 District: 1

Major Grand Ave
Minor Jefferson St

State Of Illinois
 Department of Transportation
 Bureau of Traffic

Date: 11/15/2024

SUMMARY OF TRAFFIC SURVEY

Route:	TRAFFIC FROM NORTH Jefferson St N. of: Grand Ave Going				TRAFFIC FROM SOUTH Jefferson St S. of: Grand Ave Going				TOTAL NORTH AND SOUTH	TRAFFIC FROM EAST Grand Ave E. of: Jefferson St Going				TRAFFIC FROM WEST Grand Ave W. of: Jefferson St Going				TOTAL EAST AND WEST	GRAND TOTAL
	EAST ↘	SOUTH ↓	WEST ↙	TOTAL	WEST ↙	NORTH ↑	EAST ↘	TOTAL		SOUTH ↙	WEST ←	NORTH ↘	TOTAL	NORTH ↘	EAST →	SOUTH ↙	TOTAL		
START TIME																			
AM	78	0	104	182	28	0	26	54	236	11	552	308	871	137	830	3	970	1,841	2,077
PM	122	0	165	287	22	0	14	36	323	15	705	407	1,127	181	707	7	895	2,022	2,345
SAT	189	0	251	440	20	0	10	30	470	11	394	682	1,087	303	481	5	789	1,876	2,346
55th % AM	43	0	57	100	15	0	15	30	130	6	304	169	479	75	457	2	534	1,013	1,143
55th % PM	67	0	91	158	12	0	8	20	178	8	388	224	620	100	389	4	493	1,113	1,291
55th % Sat	104	0	138	242	11	0	6	17	259	6	217	375	598	167	265	3	435	1,033	1,292

REVIEW INFORMATION

COUNTS USED: 2026 Future w/Entertainment District Traffic Volumes
 COUNT DATE(S): -
 DATA REVIEWED: 11/15/2024
 REVIEWED BY: MFM

RIGHT TURN FACTORIZATION SHEET

INTERSECTION: Grand Ave & Jefferson St
 MUNICIPALITY: Chicago

COUNTY: Cook

DIR	PEAK HOUR	MINOR STREET				CRITICAL MAINLINE APPROACH VOLUME PER LANE	BASE RIGHT TURN REDUCTION %	MAINLINE CONGESTION FACTOR %	ADJUSTED RIGHT TURN REDUCTION %	ADJUSTED RIGHT TURNS	ADJUSTED MINOR STREET VOLUME
		STREET NAME		Jefferson St							
		LEFT	THROUGH	RIGHT	APP. TOTAL						
WB	AM	78	0	104	182	430	75%	5%	70%	31	109
WB	PM	122	0	165	287	556	75%	10%	65%	58	180
WB	SAT	189	0	251	440	538	75%	10%	65%	88	277
		116	0	157	273	529	0%	10%	-10%	173	289
WB	55th AM	43	0	57	100	237	75%	0%	75%	14	60
WB	55th PM	67	0	91	158	306	75%	0%	75%	23	99
WB	55th Sat	104	0	138	242	296	75%	0%	75%	35	152

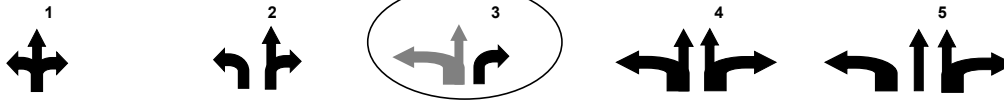
MAINLINE CONGESTION FACTORS	
VOLUMES	FACTOR %
0-399	0%
400-499	5%
500-599	10%
600-699	15%
700-799	20%
800-899	25%
900-999	30%
1000-1099	35%
1100-1199	40%
1200-1299	45%
1300-1399	50%
1400-1499	55%

REVIEW INFORMATION

COUNTS USED: 2026 Future w/Entertainment District Traffic Volumes
 COUNT DATE(S): -
 DATE REVIEWEC 11/15/2024
 REVIEWED BY: MFM

RIGHT TURN FACTORIZATION SHEET (CONT.)

LANE CONFIGURATIONS



Any configuration with an exclusive right turn lane (usually up to 600 ft. long)

PH	LEFT	THROUGH	RIGHT	TOTAL (A)	0.7A	0.35A	3T	T/3	(T+L)	(T+R)	3R	3L	T/2	T/4	L=T=R (+/-)
AM	78	0	104	182	127	64	0	0	78	104	312	234	0	0	NO
PM	122	0	165	287	201	100	0	0	122	165	495	366	0	0	NO
SAT	189	0	251	440	308	154	0	0	189	251	753	567	0	0	NO
55th AM	43	0	57	100	70	35	0	0	43	57	171	129	0	0	NO
55th PM	67	0	91	158	111	55	0	0	67	91	273	201	0	0	NO
55th Sat	104	0	138	242	169	85	0	0	104	138	414	312	0	0	NO

PH	BASE REDUCTION				
	CONFIG 1	CONFIG 2	CONFIG 3	CONFIG 4	CONFIG 5
AM	40%	60%	75%	65%	75%
PM	40%	60%	75%	65%	75%
SAT	40%	60%	75%	65%	75%
55th AM	40%	60%	75%	65%	75%
55th PM	40%	60%	75%	65%	75%
55th Sat	40%	60%	75%	65%	75%

SIGNAL WARRANT REVIEW SHEET

District #1

SRA: N/A
 YES NO

COUNTY: Cook

INTERSECTION: Grand Ave & Jefferson St
 MUNICIPALITY: Chicago

Speed Limit of Major Route 30
 Number of Lanes of Major Approach 2

Isolated Community with Population < 10, No
 Number of Lanes of Minor Approach 2

HOUR BEGIN	MAJOR STREET VOLUME (both approaches)	ADJ. MINOR STREET VOLUME (higher volume approaches)	Check any hours which meet the following Warrants			
			WARRANT 1			
			A	B	WARRANT 1 A/B: 8 hrs of both	
	100%	100%	80% of A	80% of B		
AM	1,841	109		X		X
PM	2,022	180		X	X	X
SAT	1,876	277	X	X	X	X
55th % AM	1,013	60				
55th % PM	1,113	99				X
55th % SAT	1,033	152		X		X

Volume Requirements:	Met:			
	No	Yes	No	Yes
MAJOR:	600	900	480	720
MINOR:	200	100	160	80

REVIEW INFORMATION

COUNTS USED: 2026 Future w/Entertainment District Traffic Volumes
 COUNT DATE(S): -
 DATE REVIEWED: 11/15/2024
 REVIEWED BY: MFM

Comments

WARRANT 1 YES NO N/A

Warrant 1 is met if any of the following Conditions are met:

- CONDITION A Minimum Vehicular Volume YES NO N/A
- CONDITION B Interruption of Continuous Traffic YES NO N/A
- CONDITION A/B Combination of Warrant 1 YES NO N/A

WARRANT 2 YES NO N/A

Four Hour Volume

WARRANT 3 YES NO N/A

Peak Hour Volume

WARRANT 4 YES NO N/A

Pedestrian Volume

WARRANT 5 YES NO N/A

School Crossing

WARRANT 6 YES NO N/A

Coordinated Signal System

WARRANT 7 YES NO N/A

Accidents Experience

WARRANT 8 YES NO N/A

Roadway Network

WARRANT 9 YES NO N/A

Intersection Near a Grade Crossing

Date	11/15/2024
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Location Information

Intersection	Jefferson St & Garage Entrance
City	Chicago
County	Cook
District	1

Street Information

Major Street (NB/SB)	Jefferson Street
Minor Street (EB/WB)	Parking Garage Entrance

Review Information

Counts Used	Future with Entertainment District Traffic Volumes
Count Dates	-
Date Reviewed	11/15/2024
Reviewed By	MFM

Warrant Information

Speed Limit of Major Street	30
Number of Lanes on Major	1
Number of Lanes on Minor	1
Isolated Community?	No
SRA Route Number	N/A

Jefferson St & Garage Entrance
 City: **Chicago**
 County: **Cook**
 District: **1**
Major Jefferson Street
Minor Parking Garage Entrance

State Of Illinois
 Department of Transportation
 Bureau of Traffic

Date: 11/15/2024

SUMMARY OF TRAFFIC SURVEY

Route:	TRAFFIC FROM NORTH Jefferson Street N. of: Parking Garage Entrance Going				TRAFFIC FROM SOUTH Jefferson Street S. of: Parking Garage Entrance Going				TOTAL NORTH AND SOUTH	TRAFFIC FROM EAST Parking Garage Entrance E. of: Jefferson Street Going				TRAFFIC FROM WEST Jefferson Street W. of: Jefferson Street Going				TOTAL EAST AND WEST	GRAND TOTAL
	EAST ↘	SOUTH ↓	WEST ↙	TOTAL	WEST ↙	NORTH ↑	EAST ↘	TOTAL		SOUTH ↙	WEST ←	NORTH ↗	TOTAL	NORTH ↗	EAST →	SOUTH ↘	TOTAL		
START TIME																			
Friday Casino	199	218	0	417	0	341	340	681	1,098	216	0	417	633	0	0	0	0	633	1,731
Friday 95%	189	207	0	396	0	324	323	647	1,043	205	0	396	601	0	0	0	0	601	1,644
Weekday PM	149	168	0	317	0	264	257	521	838	170	0	329	499	0	0	0	0	499	1,337
Wkday PM 95%	142	160	0	302	0	251	244	495	797	162	0	313	475	0	0	0	0	475	1,272
55th % AM	109	120	0	229	0	191	190	381	610	119	0	229	348	0	0	0	0	348	958
55th % PM	104	114	0	218	0	181	181	362	580	113	0	218	331	0	0	0	0	331	911
55th % Sat	82	92	0	174	0	148	144	292	466	94	0	181	275	0	0	0	0	275	741

REVIEW INFORMATION

COUNTS USED: Future with Entertainment District Traffic Volumes
 COUNT DATE(S): -
 DATA REVIEWED: 11/15/2024
 REVIEWED BY: MFM

RIGHT TURN FACTORIZATION SHEET

INTERSECTION: Jefferson St & Garage Entrance
 MUNICIPALITY: Chicago

COUNTY: Cook

DIR	PEAK HOUR	MINOR STREET				CRITICAL MAINLINE APPROACH VOLUME PER LANE	BASE RIGHT TURN REDUCTION %	MAINLINE CONGESTION FACTOR %	ADJUSTED RIGHT TURN REDUCTION %	ADJUSTED RIGHT TURNS	ADJUSTED MINOR STREET VOLUME
		STREET NAME		CONFIG. #							
		LEFT	THROUGH	RIGHT	APP. TOTAL						
WB	AM	216	0	417	633	681	75%	15%	60%	167	383
WB	PM	205	0	396	601	647	75%	15%	60%	158	363
WB	SAT	170	0	329	499	521	75%	10%	65%	115	285
		162	0	313	475	495	75%	5%	70%	94	256
WB	55th AM	119	0	229	348	191	75%	0%	75%	57	211
WB	55th PM	113	0	218	331	181	75%	0%	75%	55	200
WB	55th Sat	94	0	181	275	148	75%	0%	75%	45	157

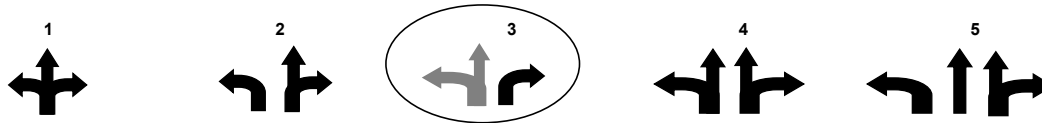
MAINLINE CONGESTION FACTORS	
VOLUMES	FACTOR %
0-399	0%
400-499	5%
500-599	10%
600-699	15%
700-799	20%
800-899	25%
900-999	30%
1000-1099	35%
1100-1199	40%
1200-1299	45%
1300-1399	50%
1400-1499	55%

REVIEW INFORMATION

COUNTS USED: Future with Entertainment District Traffic Volumes
 COUNT DATE(S): -
 DATE REVIEWED: 11/15/2024
 REVIEWED BY: MFM

LANE CONFIGURATIONS

RIGHT TURN FACTORIZATION SHEET (CONT.)



Any configuration with an exclusive right turn lane (usually up to 800 ft. long)

PH	LEFT	THROUGH	RIGHT	TOTAL (A)	0.7A	0.35A	3T	T/3	(T+L)	(T+R)	3R	3L	T/2	T/4	L=T=R (+/-)
AM	216	0	417	633	443	222	0	0	216	417	1251	648	0	0	NO
PM	205	0	396	601	421	210	0	0	205	396	1188	615	0	0	NO
SAT	170	0	329	499	349	175	0	0	170	329	987	510	0	0	NO
55th AM	119	0	229	348	244	122	0	0	119	229	687	357	0	0	NO
55th PM	113	0	218	331	232	116	0	0	113	218	654	339	0	0	NO
55th Sat	94	0	181	275	193	96	0	0	94	181	543	282	0	0	NO

PH	BASE REDUCTION				
	CONFIG 1	CONFIG 2	CONFIG 3	CONFIG 4	CONFIG 5
AM	40%	60%	75%	65%	75%
PM	40%	60%	75%	65%	75%
SAT	40%	60%	75%	65%	75%
55th AM	40%	60%	75%	65%	75%
55th PM	40%	60%	75%	65%	75%
55th Sat	40%	60%	75%	65%	75%

SIGNAL WARRANT REVIEW SHEET

District #1

SRA: N/A
 YES NO

COUNTY: Cook

INTERSECTION: Jefferson St & Garage Entrance
 MUNICIPALITY: Chicago

Speed Limit of Major Route 30
 Number of Lanes of Major Approach 1

Isolated Community with Population < 10, No
 Number of Lanes of Minor Approach 1

HOUR BEGIN	MAJOR STREET VOLUME (both approaches)	ADJ. MINOR STREET VOLUME (higher volume approaches)	Check any hours which meet the following Warrants			
			WARRANT 1			
			A	B	WARRANT 1 A/B: 8 hrs of both	
				80% of A	80% of B	
Wkdy Fri	1,098	383	X	X	X	X
Wkdy Fri 95%	1,043	363	X	X	X	X
Wkdy PM	838	285	X	X	X	X
55th % Wkdy Fri	610	211	X		X	X
55th % Wkdy Fri 95%	580	200	X		X	
55th % Wkdy PM	466	157			X	

	Met:	Yes	No	Yes	Yes
Volume Requirements:	MAJOR:	500	750	400	600
	MINOR:	150	75	120	60

REVIEW INFORMATION

COUNTS USED: Future with Entertainment District Traffic Volumes
 COUNT DATE(S): -
 DATE REVIEWED: 11/15/2024
 REVIEWED BY: MFM

Comments

WARRANT 1 YES NO N/A

Warrant 1 is met if any of the following Conditions are met:

- CONDITION A Minimum Vehicular Volume YES NO N/A
- CONDITION B Interruption of Continuous Traffic YES NO N/A
- CONDITION A/B Combination of Warrant 1 YES NO N/A

WARRANT 2 YES NO N/A

Four Hour Volume

WARRANT 3 YES NO N/A

Peak Hour Volume

WARRANT 4 YES NO N/A

Pedestrian Volume

WARRANT 5 YES NO N/A

School Crossing

WARRANT 6 YES NO N/A

Coordinated Signal System

WARRANT 7 YES NO N/A

Accidents Experience

WARRANT 8 YES NO N/A

Roadway Network

WARRANT 9 YES NO N/A

Intersection Near a Grade Crossing

TRAFFIC SIGNAL WARRANT SUMMARY

City: **Chicago**
 County: **Cook**

Engineer: **MFM**
 Date: **11/15/2024**

Major Street: **Jefferson Street**
 Minor Street: **Parking Garage Entrance**

Lanes: **1**
 Lanes: **1**

Critical Approach Speed: **30**

Volume Level Criteria

1. Is the critical speed of major street traffic > 70 km/h (40 mph)? Yes No
2. Is the intersection in a built-up area of isolated community of <10,000 population? Yes No
- If Question 1 or 2 above is answered "Yes", then use "70%" volume level 70% 100%

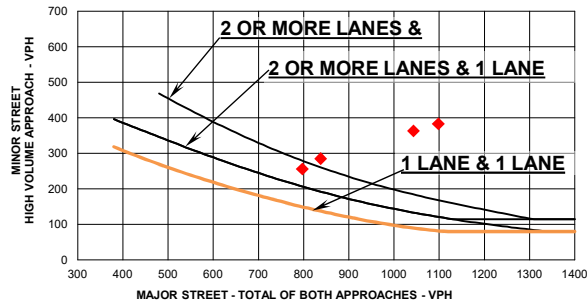
WARRANT 2 - FOUR-HOUR VEHICULAR VOLUME

If all four points lie above the appropriate line, then the warrant is satisfied.

Applicable: Yes No
 Satisfied: Yes No

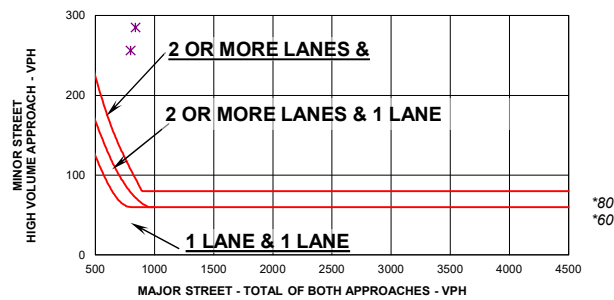
Plot four volume combinations on the applicable figure below.

FIGURE 4C-1: Criteria for "100%" Volume Level



* Note: 115 vph applies as the lower threshold volume for a minor street approach with two or more lanes and 80 vph applies as the lower threshold volume for a minor street approach with one lane.

FIGURE 4C-2: Criteria for "70%" Volume Level



* Note: 80 vph applies as the lower threshold volume for a minor street approach with two or more lanes and 60 vph applies as the lower threshold volume for a minor street approach with one lane.

FOUR HIGHEST HOURS	Volumes	
	MAJOR STREET	MINOR STREET
Friday Casino Peak Hour	1,098	383
Friday Casino 95%	1,043	363
PM Peak Hour	838	285
PM Peak Hour 95%	797	256